Appendix A Watershed Report Cards





Penetangore River Watershed

The Penetangore River consists of two major tributaries, the North and Main Penetangore Rivers, as well as two intermediate tributaries; Millarton and Kincardine Creeks. The system drains 192 square kms, falling 12.5 metres in 51.2 kms, with an average gradient of 2.2 metres per km.

The topography of this watershed is generally smooth with gentle sloping areas. It is predominantly agricultural (83%). The Town of Kincardine and the communities of Bervie and Millarton also exist here.



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testing, and so much more, is completed by staff in helping to determine the best and most applicable environmental measures to apply in each subwatershed.

Watersheds are complex systems where everything is connected.

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1078 Bruce Rd. 12, P.O. Box 150, Formosa ON N0G 1W0 Tel. 519-367-3040 Email: publicinfo@svca.on.ca www.svca.on.ca

General Information

Area

192 sq. km

Municipalities

Municipality of Kincardine, Township of Huron-Kinloss, Municipality of Brockton

Physiography

43% till plain (undrumlinized), 27% till plain (bevelled), 15% sand plain, 9% till moraine, 5% beaches and shorecliffs

Soils

61% clay loam, 18% fine to moderately coarse sandy loam, 10% silty loam, 9% other (may include small percentages of alluvium, breypan, bottomlands etc.), 2% organic material

Dams

There are no dams in the watershed

Sewage Treatment Facilities Kincardine

Woodlot Size

Limited forest cover along the lakeshore and at the back of farm lots

Land Use

83% agriculture; 11% forested; 3.9% urban

Provincially Significant Natural Areas - none

Groundwater Aquifer Sources Detroit River Group; Onondage Formation

Stream Flow (mean)

Mean annual flow - 1.63 cubic metres per second (cms)

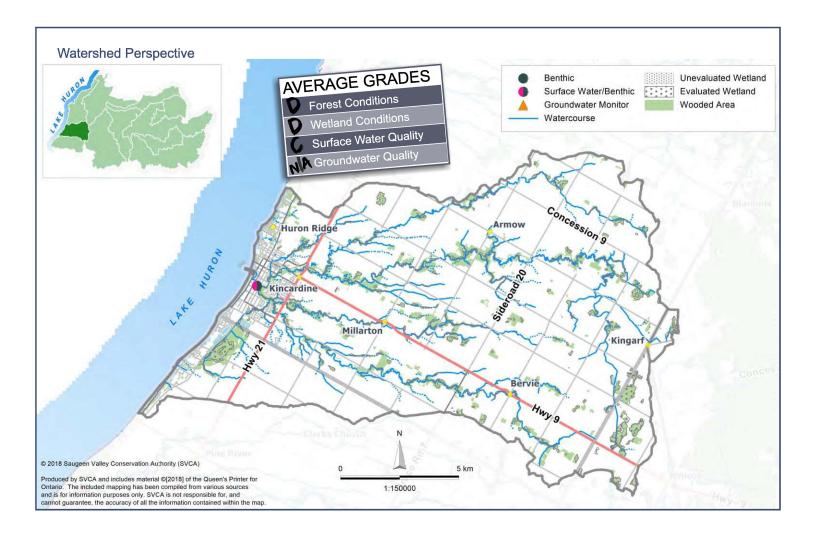
Stream Flow (low) *

7Q10 flow¹ - 0.1 cms 7Q20 flow² - less than 20 years of data

Rare Species (obtained from the National Heritage Information Centre (NHIC) Website)

Clamp-tipped Emerald, Beaked Spike-rush, Great Lakes Sand Reed, Great Lakes Wild Rye, Snapping Turtle, Eastern Meadowlark, Ram's Head Lady Slipper, Round Goby, Sand Dune Wild Rye, Bobolink

² 7Q20 - the lowest mean flow for seven consecutive days that has a 20-year recurrence interval period, or a 1 in 20 chance of occurring in any one year.



^{* 1 7}Q10 - the lowest mean flow for seven consecutive days that has a 10-year recurrence interval period, or a 1 in 10 chance of occurring in any one year.

	Indicators	2002 - 2006	2007 - 2011	2012 - 2016	Indicator Description
	Forest Cover (% of Area)	D 11.3	D 10.8	D 10.8	Forest cover is the percentage of the watershed that is forested or wooded. Environment Canada suggests that 30% forest cover is the minimum required to support healthy wildlife habitat.
Conditions	Forest Interior (% of Area)	F 1.3	F 1.0	F 0.8	Forest interior refers to the protected core area found inside a woodlot. It is the sheltered, secluded environment away from forest edges and open habitats. Environment Canada recommends that a minimum of 10% of a watershed should be interior forest cover to sustain healthy plant and animal species.
Forest C	Riparian Cover (% of Area)	B 49.0	C 29.8	C 36.3	Riparian Cover is the percentage of forested habitat along a given waterway. Environment Canada guidelines suggest that at least 75% of stream length should have 30 metre naturally vegetated buffers. Forested vegetation represents about two-thirds with the rest being marsh, meadow, and shrub thicket.
	Average Grade	D	D	D	Grade D indicates poor ecosystem conditions and overall improvements are necessary.
Wetland Conditio	Wetland Cover	No Data	D 4.0	D 4.0	Wetland cover is the percentage of existing wetland in a watershed. Environment Canada suggests that 10% wetland cover is the minimum needed for a healthy watershed. Grade D indicates poor ecosystem conditions and overall improvements are necessary.

	Indicators	2002 - 2006	2007 - 2011	2012 - 2016	Indicator Description		
Quality	Benthic Invertebrates (FBI)	D 6.02	D 5.97	D 5.83	Benthos or benthic invertebrates are bottom dwelling insects, crustaceans, worms, mollusks, and related aquatic animals that live in watercourses. They are good indicators of water quality, responding quickly to environmental stressors such as pollutants. The Modified Family Biotic Index (FBI) using New York State tolerance values provide stream health information and values ranging from 1 (healthy) to 10 (degraded).		
Surface Water Quality	Total Phosphorus (mg/L)	B 0.030	C 0.034	C 0.031	Total phosphorus is indicative of nutrient levels within a watercourse. Phosphorus is required for the growth of aquatic plants and algae, however, concentrations above the Provincial Water Quality Objective may result in unhealthy stream conditions. <i>The Provincial Water Quality Objective is</i> 0.03 mg/L.		
Surf	E. coli (cfu/100mL)	C 148	B 81	C humans, livestock, wildlife, pets and waterfowl. The Ontario Recre Water Quality Guidelines suggest that waters with less than 10c CFUs/100mL are safe for swimming.			
	Average Grade	С	С	С	Grade C indicates ecosystem conditions that need to be enhanced.		
Quality	Nitrite + Nitrate (mg/L) N/A N		N/A	N/A	Nitrates are present in water as a result of decaying plant or animal material, the use of fertilizers, domestic sewage or treated wastewater, as well as geological formations containing soluble nitrogen compounds. The Ontario Drinking Water Standard for nitrite + nitrate is 10 mg/L.		
Groundwater Quality	Chloride (mg/L)	N/A	N/A	N/A	While chloride can be naturally occurring, the presence of elevated chloride may indicate contamination from road salt, industrial discharges, or landfill leachate. The Ontario Drinking Water Standard for chloride is only for aesthetic purposes with an objective of 250 mg/L.		
Gro	Average Grade	N/A	N/A	N/A	There are no monitoring wells located within this watershed, however, other monitoring wells in the vicinity have good water quality achieving an A grade.		



Surface Water Quality

The Penetangore River scores an average grade of 'C' for surface water quality. The overall grade has stayed the same since the previous report cards. The average total phosphorus concentration is above the Provincial Water Quality Objective of 0.03 mg/L. E. coli is now above the recreational guidelines of 100 CFU/100mL., having changed from a' B' to a 'C'. At this new rating, the river would be considered unsafe for swimming.

The benthic invertebrate grade stayed the same at a 'D'. Changes in aquatuc organisms or the benthic invertebrate community are seen as early indicators of changes in water quality. Efforts must continue to encourage landowners and the agricultural community to preserve and improve natural land cover.

Groundwater Quality

There are no groundwater monitoring wells located within this watershed.

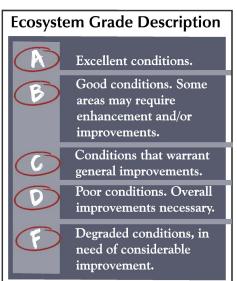
Forest Conditions

This watershed falls short of meeting the Environment Canada guidelines of 30% forest cover, with an average grade of 'D'. Forest cover and forest interior grades did not change with a 'D' and an 'F', respectively. There are a limited number of small, fragmented forests, many of which exist along the banks of streams. Riparian forested cover scored a 'C' grade. The recommendation is that 50% of the 30 metre wide riparian zone should have forest cover. The Penetangore River Watershed has only 36.3% of the riparian zone forested. Tree planting along riparian zones and on marginal farmland should be considered to improve forest conditions and existing forests should be protected.

Wetland Conditions

This report card summarizes the conditions of all wetlands within this watershed, which score a 'D' grade at only 4.0% wetland cover. This is below the Environment Canada recommendation of 10% as the minimum standard. Almost all of the wetlands have been cleared for agricultural land. It would be highly recommended to allow low lying or wet areas to naturalize and to protect existing ones. These are key areas in terms of overall watershed health.

The wetland evaluation system was created to inform Ontario land use planning process. Under the Planning Act, provincially significant wetlands are protected from development and alteration.





- Saugeen Conservation aims to improve watershed health through virtually all its programs.
- Saugeen Conservation is a key player in providing assistance and technical expertise to local groups, committees, ministries etc. that work to improve the local environment.
- Through Saugeen Conservation's tree planting efforts and Ontario's 50 Million Tree Program, a total of 45,100 trees were planted in this watershed.
- The Penetangore Watershed Group (established in 2011), plants approximately 3,000 trees each year in this watershed. This group is also involved in habitat development as well as habitat rehabilitation and invasive species removal. Their focus is to increase tree cover in the watershed, especially along local waterways to help improve water quality.
- √The Huron Fringe Field Naturalists work to preserve wildlife and natural habitat and to promote public interest and knowledge of the natural history in this area. In so doing, they conduct public hikes and workshop, participate in tree planting efforts, bird house construction and bird counts.
- ✓ The Ontario Steelheader's Association and the Lake Huron Fishing Club release adult rainbow trout into this river system on an annual basis. (This was discontinued in 2016.)
- ✓ Saugeen Conservation works closely with **local agricultural organizations** to provide ongoing workshops and seminars for farmers on a variety of different conservation topics.
- ✓ Grey Bruce Sustainability Network works closely with Saugeen
 Conservation on several different environmental and educational
 projects.
- √The Bruce Grey Woodlands Association hosts various workshops and tours on forestry related topics.







- The Forest Health Collaborative helps to educate municipalities and the public on forest health issues.
- Stewardship Grey Bruce offers funding and technical support for landowners in the watershed interested in completing habitat enhancement projects.
- The Lake Huron Fishing Club (with funding from Bruce Power), works with local schools in setting up fish aquariums to educate students about the importance of a healthy fishery.
- ✓ Saugeen Conservation offers over **50 different hands-on environmental programs** to over 10,000 children annually, including the Grey Bruce Children's Water Festival and the Bruce Grey Forest Festival.
- ✓ Staff have implemented the Yellow Fish Road Program, (a program of Trout Unlimited Canada), which educates students and the public about storm drains and how they are corridors to local rivers and streams.
- Grey-Bruce ALUS program recognizes land stewardship and assists farmers in implementing and funding projects to produce ecosystem services. ALUS aims to improve the biodiversity on the agricultural landscape.
- Environmental self assessments are now available for the rural non-farm landowner with the release of The Rural Landowner Stewardship Guide for the Lake Huron Watershed. This guide provides a framework for landowners to evaluate their property and help determine best management practices.
- The Lake Huron Centre for Coastal Conservation initiates a number of programs aimed at protecting and restoring Lake Huron's shoreline's coastal environment and promoting a healthy coastal ecosystem



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Bruce Power









































WATERSHED REPORT CARD



Lake Fringe Watershed

The Lake Fringe area is a narrow strip of land along Lake Huron stretching from north of Kincardine to Southampton.

This watershed is 254 square kms, with a number of small tributaries flowing directly into Lake Huron. The main tributaries are Lorne, Andrews, Tiverton and Underwood Creeks, as well as the Little Sauble River.

This watershed area is mainly agricultural (60%), with forested sections along the lakeshore where intensive development exists. It includes the communities of Southampton, Port Elgin, and Tiverton. Bruce Nuclear Power Development (BNPD) also exists here.



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testing, and so much more, is completed by staff in helping to determine the best and most applicable environmental measures to apply in each subwatershed.

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General Information

Area

254 sq. km

Municipalities

Municipality of Kincardine, Town of Saugeen Shores

Physiography

44% till plain (drumlinized), 39% sand plain, 15% beaches and shore cliffs, 2% peat and muck

Soils

23% silty loam, 18% clay loam, 16% fine to moderately coarse sandy loam, 12% silty clay, 11% medium to moderately fine loam, 6% organic material, 6% other (may include small percentages of alluvium, breypan, bottomlands etc), 6% coarse sandy loam and loamy sand, and 0.3% gravel

Dams

There are no dams in the watershed

Sewage Treatment Facilities

None

Woodlot Size

Large woodlots with forest interior along the lakeshore with the rest of the watershed limited to small fragmented forests at the back of farm lots

Land Use

60% agriculture; 29% forested; 6.1% urban

Provincially Significan Natural Areas - Scott Point, Baie duDore, MacGregor Point Wetland Complex

Groundwater Aquifer Sources

Salina Formation, Bass Island Formation, Bois Blanc Formation; Oriskany Formation, Detroit River Group; Onondage Formation, Lucas Formation, Glaciolacustrine Formation

Stream Flow (mean) N/A

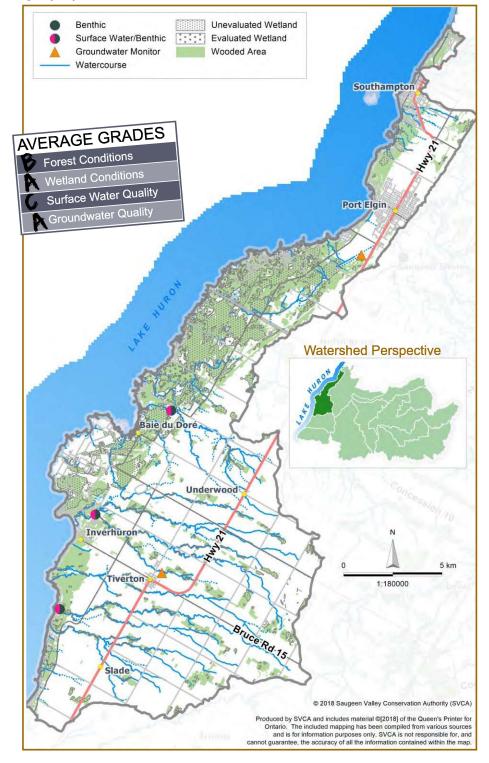
Stream Flow (low) * N/A

Rare Species (obtained from the National Heritage Information Centre (NHIC) Website)

American Beach Grass, Barn Swallow, Beach-dune Tiger Beetle, Beaked Spikerush, Black Meadowlark, Bobolink, Brushed-tipped Emerald, Butternut, Eastern Meadowlark, Eastern Red Damsel, Eastern Milksnake, Eastern Ribbonsnake, Lake Sturgeon, Hill's Pondweed, Pitcher's Thistle, Great Egret, Greene's Rush, Loggerhead Shrike, Dwarf Lake Iris, Great Lakes Sand Reed, Great Lakes Wild Rye, Greene's Rush, Green-striped Darner, Low Nutrush, Neglected Milk-vetch, Northern Brook Lamprey, Ocellated Darner, Prairie Dropseed, Queensnake, Ram's-head Lady Slipper, Red-shouldered Hawk, Ram's-head

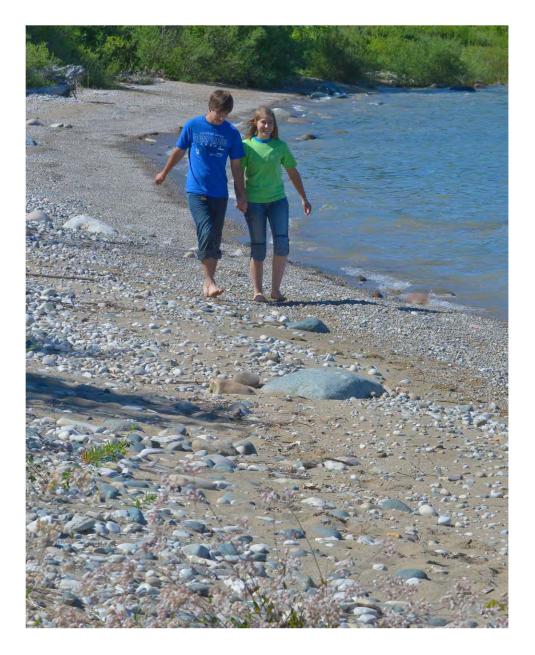
Lady's Slipper, Rough Dropseed, Sand-dune Wildrye, Stiff Yellow Flax, Small White Lady Slipper, Tuberous Indian-plantain, Stiff Gentian, White Perch, Williamson's Emerald, Snapping Turtle, Threespine Stickleback

- * ¹ 7Q10 the lowest mean flow for seven consecutive days that has a 10-year recurrence interval period, or a 1 in 10 chance of occurring in any one year.
- ² 7Q20 the lowest mean flow for seven consecutive days that has a 20-year recurrence interval period, or a 1 in 20 chance of occurring in any one year.



	Indicators	2002 - 2006	2007 - 2011	2012 - 2016	Indicator Description
	Forest Cover (% of Area)	B 28.8	B 28.5	B 28.4	Forest cover is the percentage of the watershed that is forested or wooded. Environment Canada suggests that 30% forest cover is the minimum required to support healthy wildlife habitat.
orest Conditions	Forest Interior (% of Area)	B 10.5	B 8.6	B 8.6	Forest interior refers to the protected core area found inside a woodlot. It is the sheltered, secluded environment away from forest edges and open habitats. Environment Canada recommends that a minimum of 10% of a watershed should be interior forest cover to sustain healthy plant and animal species.
Forest (Riparian Cover (% of Area)	C 37.0	C 29.5	C 33.6	Riparian Cover is the percentage of forested habitat along a given waterway. Environment Canada guidelines suggest that at least 75% of stream length should have 30 metre naturally vegetated buffers. Forested vegetation represents about two-thirds with the rest being marsh, meadow, and shrub thicket.
	Average Grade	В	В	В	Grade B indicates good ecosystem conditions. Some areas may require enhancement.
Wetland Conditions	Wetland Cover	No Data	A 14.2	A 14.2	Wetland cover is the percentage of existing wetland in a watershed. Environment Canada suggests that 10% wetland cover is the minimum needed for a healthy watershed. Grade A indicates excellent ecosystem conditions and protection may be required. Some areas may require enhancement to maintain this level of quality.

	Indicators	2002 - 2006	2007 - 2011	2012 - 2016	Indicator Description		
. Quality	Benthic Invertebrates (FBI)	B 4.27	D D 6.17 6.07		Benthos or benthic invertebrates are bottom dwelling insects, crustaceans, worms, mollusks, and related aquatic animals that live in watercourses. They are good indicators of water quality, responding quickly to environmental stressors such as pollutants. The Modified Family Biotic Index (FBI) using New York State tolerance values provide stream health information and values ranging from 1 (healthy) to 10 (degraded).		
Surface Water Quality	Total Phosphorus (mg/L)	A 0.010	B 0.025	B 0.025	Total phosphorus is indicative of nutrient levels within a watercourse. Phosphorus is required for the growth of aquatic plants and algae, however, concentrations above the Provincial Water Quality Objective may result in unhealthy stream conditions. The Provincial Water Quality Objective is 0.03 mg/L.		
Sur	E. coli (cfu/100mL)	B 68	B 69	B 59	E. coli originate from the wastes of warm blooded animals, including humans, livestock, wildlife, pets and waterfowl. The Ontario Recreational Water Quality Guidelines suggest that waters with less than 100 CFUs/100mL are safe for swimming.		
	Average Grade	В	С	С	Grade C indicates ecosystem conditions that need to be enhanced.		
Quality	Nitrite + Nitrate (mg/L)	No Data	A 0.06	A 0.04	Nitrates are present in water as a result of decaying plant or animal material, the use of fertilizers, domestic sewage or treated wastewater, as well as geological formations containing soluble nitrogen compounds. The Ontario Drinking Water Standard for nitrite + nitrate is 10 mg/L.		
Groundwater Quality	Chloride (mg/L)	No Data	A 12.97	A 13.98	While chloride can be naturally occurring, the presence of elevated chloride may indicate contamination from road salt, industrial discharges, or landfill leachate. The Ontario Drinking Water Standard for chloride is only for aesthetic purposes with an objective of 250 mg/L.		
Ģ	Average Grade	No Data	А	Α	Grade A indicates excellent ecosystem conditions and protection may be required. Some areas may require enhancement to maintain this level o quality.		



Forest Conditions

With an average grade of 'B' for forest conditions, the Lake Fringe Watershed does not meet the Environment Canada guidelines of 30% forest cover and 10% forest interior. Forest Cover maintained a 'B' grade from the previous Report Cards while Forest Interior remained a 'B'. The grade for riparian cover scored a 'C' grade. The recommendation is that 50% of the 30 metre wide riparian zone should have forest cover. The Lake Fringe watershed has only 33.6% of the riparian zone forested. Tree planting along riparian zones, on dormant fields, areas too wet to farm, and on marginal farmland should be considered to ensure the forest conditions are maintained or improved.

Wetland Conditions

This report card summarizes the conditions of all wetlands. The Lake Fringe Watershed scores an 'A' grade with 14.2% wetland cover. This is just above the Environment Canada recommendation of 10% (minimum required for a healthy watershed).

It would be advisable to allow low lying or wet areas to naturalize. These are key areas and allowing them to regenerate will help to improve wetland scores.

The wetland evaluation system was created to protect important wetlands valued at a provincial level. Under the Planning Act, provincially significant wetlands are protected from development and alteration.

Surface Water Quality

This watershed scores an average grade of 'C' for *surface* water quality, indicating that ecosystem conditions need to be enhanced. The overall grade has remained a 'C' since the last set of report cards.

The average total phosphorus concentration is below the provincial water quality objective of 0.03 mg/L. The average E. coli is also below the recreational guidelines of 100 FU/100mL. The benthic invertebrate grade remained a "D' but is still a cause for concern. These low grades in the benthic invertebrate community are seen as early indicators of water quality deterioration. Efforts should continue to encourage landowners and the agricultural community to preserve and improve natural land cover. In addition to managing current land use practices, climate change and invasive species also pose significant threats.

Groundwater Quality

Groundwater quality in the two monitoring wells in this watershed continues to be excellent. It should be noted that groundwater aquifers do not conform to watershed boundaries but flow in an east to west direction through the watershed. Of note, is that there have been exceedences of the Ontario Drinking Water Standards for sodium during this study period at the Tiverton well.





- ✓ Saugeen Conservation aims to improve watershed health through virtually all its programs.
- Saugeen Conservation is a key player in providing assistance and technical expertise to local groups, committees, ministries etc. that work to improve the local environment.
- Through Saugeen Conservation's tree planting efforts and Ontario's 50 Million Tree Program, a total of 45,100 trees were planted in this watershed, during this report period.
- The Lake Huron Fishing Club actively stocks steelhead salmon in the Saugeen River. The club operates two hatcheries (Kincardine and Port Elgin), rearing young fish to help supplement the fishery. The School Salmon Hatchery program has grown from 10 Schools in 2012 to 47 Schools by 2016 with all the salmon being released into the Saugeen or Penetangore Rivers and fully funded by Bruce Power.
- SauGREEN for the Environment is a local environmental community group focused in the Saugeen Shores area. They implement various environmental initiatives including waste diversion, rain barrels, tree planting, Tall Tree Initiative and other eco-friendly projects.
- Friends of MacGregor Provincial Park hosts the annual Huron Fringe Birding Festival which attracts hundreds of participants each year.
- Saugeen Conservation works closely with **local agricultural organizations** and institutions to provide ongoing workshops and seminars for farmers on a variety of different topics relating to environmental and farm health, including soil conservation, cover crops, no-till farming, etc.
- ✓ Local tourism groups and organizations play a large part in promoting the wealth of natural resources in the Saugeen Watershed.

Saugeen Conservation works closely with each of them in promoting the sustainability of the environment and its precious resources, including this area.

- Grey Bruce Sustainability Network works closely with Saugeen Conservation on several different environmental projects, ranging from green development to rain gardens, educational programs, river cleanup operations, seminars and more.
- The Bruce Grey Woodlands Association hosts various workshops and tours on forestry related topics. They also organize the annual Grey Bruce Woodlot Conference focusing on important forest-related issues and research.





- The Forest Health Collaborative includes forestry and tree professionals throughout Grey and Bruce Counties. Its objectives include educating municipalities and the public on forest health issues.
- The Lake Huron Centre for Coastal Conservation works with Saugeen Conservation, in providing expertise relating to Lake Huron and shoreline issues. They specialize in research, technical advice, education programs, public outreach, stewardship efforts and much more.
- Stewardship Grey Bruce offers support funding and technical support for landowners in the watershed who are interested in completing habitat enhancement projects focused on focused on water quality improvement.
- Special attention is provided in this watershed with regard to the combat and control of the invasive Phragmites australis plant that has taken over much of the shoreline area. Various groups, as well as the local municipalities have been active in this program. Enbridge Inc., and Bruce Power have been instrumental in assisting with funding and manpower.
- Saugeen Conservation offers over 50 different hands-on conservation education programs designed to get students outdoors and learning about the natural environment. Thirty different programs are provided by Saugeen Conservation and Bruce Power at the Bruce Nuclear Power Development, (BNPD), complex free of charge to all local schools. In addition, local schools attend both the Bruce Grey Forest Festival and the Grey Bruce Children's Water Festival.
- Healthy Lake Huron is an initiative of local environmental organizations, including Saugeen Conservation. Together they coordinate actions to protect and improve overall water quality along the southeast shores of Lake Huron.
- Environmental self assessments are now available for the rural non-farm landowner with the release of The Rural Landowner Stewardship Guide for the Lake Huron Watershed. This guide provides a framework for landowners to evaluate their property and help determine best management practices.

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Appendix B Species at Risk List – Bruce County

Table A.1 – Federal and Provincial Species at Risk with Potential Habitat in Municipality of Kincardine

Туре	Species Common Name	Species Scientific Name	Federal Status	Provincial Status	Suitable Habitat in the Study Area	Reported within Study Area
Bird	Bald Eagle	Haliaeetus leucocephalus	Not Applicable	Special Concern	No	No
Bird	Bank Swallow	Riparia riparia	Threatened	Threatened	Yes	Yes
Bird	Barn Swallow	Hirundo rustica	Threatened	Threatened	Yes	Yes
Bird	Black Tern	Chlidonias niger	Not Applicable	Special Concern	Yes	Yes
Bird	Bobolink	Dolichonyx oryzivorus	Threatened	Threatened	Yes	Yes
Bird	Eastern Wood-pewee	Contopus virens	Special Concern	Special Concern	Yes	Yes
Bird	Canada Warbler	Cardellina Canadensis	Threatened	Special Concern	Yes	Yes
Bird	Chimney Swift	Chaetura pelagica	Threatened	Threatened	No	Yes
Bird	Common Nighthawk	Chordeiles minor	Threatened	Special Concern	Yes	Yes
Bird	Eastern Meadowlark	Sturnella magna	Threatened	Threatened	Yes	Yes
Bird	Eastern Whip-poor-will	Caprimulgus vociferus	Threatened Threatened		No	Yes
Bird	Grasshopper Sparrow	Ammodramus savannarum	Special Concern	Special Concern	Yes	Yes
Bird	Least Bittern	Ixobrychus exilis	Threatened	Threatened	Yes	Yes
Bird	Loggerhead Shrike	Lanius Iudovicianus	Endangered	Endangered	Yes	No

Туре	Species Common Name	Species Scientific Name	Federal Status	Provincial Status	Suitable Habitat in the Study Area	Reported within Study Area
Bird	Peregrine Falcon	Falco peregrines	Special Concern	Special Concern	Yes	No
Bird	Red-headed Woodpecker	Melanerpes erythrocephalus	Threatened	Threatened	Yes	Yes
Bird	Wood Thrush	Hylocichla mustelina	Threatened	Special Concern	Yes	Yes
Fish	Shortnose Cisco	Coregonus reighardi	Endangered	Endangered	No	No
Reptile and Amphibian	Eastern Ribbonsnake (Great Lakes population)	Thamnophis sauritius	Special Concern	Special Concern	Yes	Yes
Reptile and Amphibian	Midland Painted Turtle	Chrysemys picta marginata	Special Concern	Not Applicable	Yes	Yes
Reptile and Amphibian	Eastern Milksnake	Lampropeltis triangulum	Special Concern	Not Applicable	Yes	Yes
Reptile and Amphibian	Snapping Turtle	Chelydra serpentina	Special Concern	Special Concern	Yes	Yes
Insect	Monarch	Danaus plexippus	Special Concern	Special Concern	Yes	Yes
Insect	Yellow Banded Bumble Bee	Bombus terricola	Special Concern	Special Concern	Yes	Yes
Mammal	Eastern Small-footed Myotis	Myotis leibii	Not Applicable	Endangered	Yes	No
Mammal	Little Brown Myotis	Myotis lucifugus	Endangered	Endangered	Yes	Yes
Mammal	Northern Myotis	Myotis septentrionalis	Endangered	Endangered	Yes	Yes
Mammal	Tri-coloured Bat	Perimyotis subflavus	Endangered	Endangered	Yes	No
Plant and Lichen	American Chestnut	Castanea dentata	Endangered	Endangered	Yes	Yes
Plant and Lichen	Butternut	Juglans cinerea	Endangered	Endangered	Yes	Yes

Туре	Species Common Name	Species Scientific Name	Federal Status	Provincial Status	Suitable Habitat in the Study Area	Reported within Study Area
Plant and Lichen	Dwarf Lake Iris	Iris lacustris	Special Concern	Special Concern	Yes	Yes
Plant and Lichen	Hill's Thistle	Cirsium hillii	Threatened	Threatened	No	No
Plant and Lichen	Hill's Pondweed	Potamogeton hillii	Special Concern	Special Concern	No	No
Plant and Lichen	Pitcher's Thistle	Cirsium pitcheri	Special Concern	Threatened	Yes	Yes

Appendix C

Kincardine WaterCAD® Modelling Information

Municipality of Kincardine WaterCAD Modelling for Master Plan Update Calculations and Notes for Kincardine

Job #: 22128

Date: October 14, 2022

Revised:

1.0 Background

The Municipality of Kincardine is completing a water and wastewater Master Plan update process. The water supply component will include a review of servicing existing development and future development for peak hour and maximum day + fire flow demands. The purpose of these notes is to summarize data used to update the WaterCAD model, and the results of that modelling for the community of Kincardine. The model was originally created as part of BMROSS project 16130.

2.0 Analysis & Model Data

2.1 Data

Reference	<u>Item</u>		
	Existing avg. day demand	t	38.1 L/s
		=	3294 m³/d
22128	Existing max. day deman	d	80.5 L/s
22120		=	6954 m³/d
	From above, max. day fac	ctor	2.11
22128	Kincardine town pop. (202	21)	7728 persons
MECP	Peak hour factors 3,001 to 10,000 population 10,001 to 25,000 population		3.00 2.85
	Existing peak hour deman	nd =	114.38 L/s 9882 m³/d
	WTP High Lift		
	Pump rating (HLP1, HLP3	3)	130 L/s
DWWD		['] @	79 m TDH
DWWP	Pump rating (HLP2)		82 L/s
		@	79 m TDH
	Clearwell volume		4120 m ³
77000	Clearwell midpoint		178.7 mASL
77066	Pump discharge		182.4 mASL
	HLP off (tower level)		39.2 m
Town info		=	247.0 mASL
	HLP on (tower level)		37.2 m
		=	245.0 mASL

DWWP/ 78011/ 19035	Standpipe Total volume Usable volume Diameter Top operating range Bottom operating range Top of foundation BPS target discharge pressure	3360 n 3190 n 10.5 n 247.00 n 243.00 n 207.50 n 360.00 k	n ³ n nASL nASL nASL	
DWWP/ 78011/ 19035	Standpipe BPS Pump rating @ @ Floor elevation	44 L 14.5 n 85 L 17.5 n 170 L 17.5 n 207.4 n	n TDH /s n TDH /s n TDH	
MECP Guide	Pipe C-factors <u>Pipe Dia. (mm)</u> 150 200-250 300-600 >600	<u>C</u> 100 110 120 130		
MECP Guide	Normal operating pressure range Normal operating pressure minin Fire flow system pressure minim Maximum allowable system pres	num um	350 to 480 kPa 275 kPa 140 kPa 700 kPa	

2.2 Water Demands by Junction

(a) Existing Conditions

356
0.107 L/s
0.226 L/s
0.321 L/s

See attached map for area junctions. Based on the data above, dividing the average day demand, maximum day demand, and peak hour demands for the system over the total number of existing model junctions would result in per junction demands of approximately 0.11 L/s, 0.23 L/s, and 0.32 L/s, respectively. Based on 2013-2015 data provided by the Municipality, the 20 largest water users connected to the system had average day demands ranging from 0.08 to 0.67 L/s each.

Design fire flow demands will vary from about 50 L/s for residential areas to 150 L/s or greater in ICI areas. Considering the relatively small demand associated with consumption as compared to fire flow, and the fact that there are few customers with significant water demand, the total system demand is distributed evenly over all model junctions.

(b) Future Conditions

85700

47300

Demands for existing development are left unchanged, and the incremental future demand for development areas is applied to the nearest model junctions within or adjacent to the development lands. Demand associated with infill is applied evenly across all junctions.

Development areas, including commitments as well as development areas for which approval is pending, are summarized below. The number of ERUs for each was established based on approved or submitted development plans where known.

Maximum day demand per ERU	1.71 m ³ /unit/d
Design water demand for commercial area	28.0 m ³ /ha/d
Maximum day factor	2.11
Peak hour factor	3.00
Peak hour demand per ERU	2.43 m³/unit/d

Developm	Development Name		Projected No. of ERUs	Maximum Day Demand (L/s)	Peak Hour Demand (L/s)	Model Junctions to Apply Demand
Kincardine Infil	I		365	7.22	10.27	All
West Ridge on the Lake			383	7.58	10.77	J-1450, J-1451, J-1455, J-1460, J-1461, J-1475, J-1480
Brown Subdivis			245	4.85	6.89	J-155
CR Developers			82	1.62	2.31	J-430
Battler Subdivis			30	0.59	0.84	J-1330
Shepherd Subo			36	0.71	1.01	J-1335
Golf Links Tow	nhouses		6.8	0.13	0.19	J-1330
Brigadoon			150	2.97	4.22	J-155
Inverhuron - Su	undance		60	1.19	1.69	J-1690
OPF Lands			1006	19.91	28.29	J-1360, J-1345, J-1315, J-1220
9 &21 Business Park			500	9.90	14.06	J-805, J-765, J-760, J-800, J-755, J-740, J-750, J-795, J-735, J-745, J-640, J-635
1182 Queen Street		12	0.24	0.34	J-1305	
O'Malley (Sutto	on/Gary Street)		32	0.64	0.91	J-3015
7 Mount Forest			25	0.49	0.69	J-1315
Concession 2 I	ndustrial Park		292	5.79	8.22	J-1740
869 Kincardine	Ave Trailer Pa	rk	88	1.73	2.46	J-430
7 Millennium W	/ay		173	3.42	4.87	J-770

denotes commercial land; others are residential

		Existing Futur			ıre, Incl. Infill	
		Maximum	Peak Hour	Maximum	Peak Hour	
Model	Elevation	Day Demand	Demand	Day Demand	Demand	
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)	
J-5	179.9	0.226	0.321	0.246	0.350	
J-10	190.0	0.226	0.321	0.246	0.350	
J-15	191.5	0.226	0.321	0.246	0.350	
J-20	195.6	0.226	0.321	0.246	0.350	
J-25	198.5	0.226	0.321	0.246	0.350	
J-30	195.0	0.226	0.321	0.246	0.350	
J-35	194.1	0.226	0.321	0.246	0.350	
J-40	193.1	0.226	0.321	0.246	0.350	
J-45	194.8	0.226	0.321	0.246	0.350	
J-50	177.5	0.226	0.321	0.246	0.350	
J-55	181.1	0.226	0.321	0.246	0.350	
J-60	192.7	0.226	0.321	0.246	0.350	
J-65	188.4	0.226	0.321	0.246	0.350	
J-70	188.4	0.226	0.321	0.246	0.350	
J-75	188.0	0.226	0.321	0.246	0.350	
J-80	192.0	0.226	0.321	0.246	0.350	
J-85	198.4	0.226	0.321	0.246	0.350	
J-90	195.4	0.226	0.321	0.246	0.350	
J-95	195.9	0.226	0.321	0.246	0.350	
J-100	198.4	0.226	0.321	0.246	0.350	
J-100 J-105	198.4					
		0.226	0.321	0.246	0.350	
J-110	195.7	0.226	0.321	0.246	0.350	
J-115 J-120	195.2 196.7	0.226	0.321	0.246	0.350	
		0.226	0.321	0.246	0.350	
J-125	199.9	0.226	0.321	0.246	0.350	
J-130	199.4	0.226	0.321	0.246	0.350	
J-135	198.3	0.226	0.321	0.246	0.350	
J-140	198.7	0.226	0.321	0.246	0.350	
J-145	200.1	0.226	0.321	0.246	0.350	
J-150	200.1	0.226	0.321	0.246	0.350	
J-155	200.7	0.226	0.321	8.064	8.168	
J-160	198.5	0.226	0.321	0.246	0.350	
J-165	196.9	0.226	0.321	0.246	0.350	
J-170	198.0	0.226	0.321	0.246	0.350	
J-175	199.0	0.226	0.321	0.246	0.350	
J-180	198.5	0.226	0.321	0.246	0.350	
J-185	194.9	0.226	0.321	0.246	0.350	
J-190	195.6	0.226	0.321	0.246	0.350	
J-195	195.5	0.226	0.321	0.246	0.350	
J-200	197.1	0.226	0.321	0.246	0.350	
J-205	196.7	0.226	0.321	0.246	0.350	
J-210	197.8	0.226	0.321	0.246	0.350	
J-215	197.4	0.226	0.321	0.246	0.350	
J-220	197.4	0.226	0.321	0.246	0.350	
J-225	197.3	0.226	0.321	0.246	0.350	
J-230	196.7	0.226	0.321	0.246	0.350	
J-235	200.0	0.226	0.321	0.246	0.350	
J-240	200.2	0.226	0.321	0.246	0.350	
J-245	181.7	0.226	0.321	0.246	0.350	
J-250	192.0	0.226	0.321	0.246	0.350	
J-255	191.0	0.226	0.321	0.246	0.350	

		Existing		Future, Incl. Infill		
		Maximum	Peak Hour	Maximum	Peak Hour	
Model	Elevation	Day Demand	Demand	Day Demand	Demand	
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)	
J-260	197.6	0.226	0.321	0.246	0.350	
J-265	191.5	0.226	0.321	0.246	0.350	
J-270	191.9	0.226	0.321	0.246	0.350	
J-275	194.1	0.226	0.321	0.246	0.350	
J-280	200.7	0.226	0.321	0.246	0.350	
J-285	197.5	0.226	0.321	0.246	0.350	
J-290	197.4	0.226	0.321	0.246	0.350	
J-295	200.8	0.226	0.321	0.246	0.350	
J-300	200.2	0.226	0.321	0.246	0.350	
J-305	198.2	0.226	0.321	0.246	0.350	
J-310	199.6	0.226	0.321	0.246	0.350	
J-315	198.2	0.226	0.321	0.246	0.350	
J-320	199.0	0.226	0.321	0.246	0.350	
J-325	191.0	0.226	0.321	0.246	0.350	
J-330	196.3	0.226	0.321	0.246	0.350	
J-335	192.6	0.226	0.321	0.246	0.350	
J-340	194.6	0.226	0.321	0.246	0.350	
J-345	194.3	0.226	0.321	0.246	0.350	
J-350	199.7	0.226	0.321	0.246	0.350	
J-355	201.5	0.226	0.321	0.246	0.350	
J-360	201.6	0.226	0.321	0.246	0.350	
J-365	194.3	0.226	0.321	0.246	0.350	
J-370	194.8	0.226	0.321	0.246	0.350	
J-375	196.4	0.226	0.321	0.246	0.350	
J-380	198.0	0.226	0.321	0.246	0.350	
J-385	196.1	0.226	0.321	0.246	0.350	
J-390	195.8	0.226	0.321	0.246	0.350	
J-395	195.8	0.226	0.321	0.246	0.350	
J-400	195.2	0.226	0.321	0.246	0.350	
J-405	200.1	0.226	0.321	0.246	0.350	
J-410	196.8	0.226	0.321	0.246	0.350	
J-415	192.0	0.226	0.321	0.246	0.350	
J-420	191.5	0.226	0.321	0.246	0.350	
J-425	195.8	0.226	0.321	0.246	0.350	
J-430	199.1	0.226	0.321	3.603	3.707	
J-435	195.9	0.226	0.321	0.246	0.350	
J-440	195.5	0.226	0.321	0.246	0.350	
J-445	193.7	0.226	0.321	0.246	0.350	
J-450	192.4	0.226	0.321	0.246	0.350	
J-455	195.2	0.226	0.321	0.246	0.350	
J-460	196.5	0.226	0.321	0.246	0.350	
J-465	196.8	0.226	0.321	0.246	0.350	
J-470	193.0	0.226	0.321	0.246	0.350	
J-475	195.0	0.226	0.321	0.246	0.350	
J-480	196.7	0.226	0.321	0.246	0.350	
J-485	197.8	0.226	0.321	0.246	0.350	
J-490	201.7	0.226	0.321	0.246	0.350	
J-495	189.4	0.226	0.321	0.246	0.350	
J-500	196.5	0.226	0.321	0.246	0.350	
J-505	196.6	0.226	0.321	0.246	0.350	
J-510	196.7	0.226	0.321	0.246	0.350	

		Existing		Future, I	ncl. Infill
		Maximum	Peak Hour	Maximum	Peak Hour
Model	Elevation	Day Demand	Demand	Day Demand	Demand
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)
J-515	196.8	0.226	0.321	0.246	0.350
J-520	198.4	0.226	0.321	0.246	0.350
J-525	197.4	0.226	0.321	0.246	0.350
J-530	198.0	0.226	0.321	0.246	0.350
J-535	195.1	0.226	0.321	0.246	0.350
J-540	196.1	0.226	0.321	0.246	0.350
J-545	197.7	0.226	0.321	0.246	0.350
J-550	197.1	0.226	0.321	0.246	0.350
J-555	198.7	0.226	0.321	0.246	0.350
J-560	197.4	0.226	0.321	0.246	0.350
J-565	198.4	0.226	0.321	0.246	0.350
J-570	198.4	0.226	0.321	0.246	0.350
J-575	199.1	0.226	0.321	0.246	0.350
J-580	197.7	0.226	0.321	0.246	0.350
J-585	197.4	0.226	0.321	0.246	0.350
J-590	198.7	0.226	0.321	0.246	0.350
J-595	197.5	0.226	0.321	0.246	0.350
J-600	197.6	0.226	0.321	0.246	0.350
J-605	202.2	0.226	0.321	0.246	0.350
J-610	201.1	0.226	0.321	0.246	0.350
J-615	203.0	0.226	0.321	0.246	0.350
J-620	199.4	0.226	0.321	0.246	0.350
J-625	204.0	0.226	0.321	0.246	0.350
J-630	204.2	0.226	0.321	0.246	0.350
J-635	205.4	0.226	0.321	1.071	1.175
J-640	209.5	0.226	0.321	1.071	1.175
J-645	199.1	0.226	0.321	0.246	0.350
J-650	203.9	0.226	0.321	0.246	0.350
J-655	204.7	0.226	0.321	0.246	0.350
J-660	205.5	0.226	0.321	0.246	0.350
J-665	205.7	0.226	0.321	0.246	0.350
J-670	201.7	0.226	0.321	0.246	0.350
J-675	205.7	0.226	0.321	0.246	0.350
J-680	205.1	0.226	0.321	0.246	0.350
J-685	205.9	0.226	0.321	0.246	0.350
J-690	206.0	0.226	0.321	0.246	0.350
J-695	206.4	0.226	0.321	0.246	0.350
J-700	205.1	0.226	0.321	0.246	0.350
J-705	205.4	0.226	0.321	0.246	0.350
J-710	206.9	0.226	0.321	0.246	0.350
J-715	206.9	0.226	0.321	0.246	0.350
J-720	206.3	0.226	0.321	0.246	0.350
J-725	208.4	0.226	0.321	0.246	0.350
J-730	209.5	0.226	0.321	0.246	0.350
J-735	205.4	0.226	0.321	1.071	1.175
J-740	206.7	0.226	0.321	1.071	1.175
J-745	207.3	0.226	0.321	1.071	1.175
J-750	206.9	0.226	0.321	1.071	1.175
J-755	207.8	0.226	0.321	1.071	1.175
J-760	212.8	0.226	0.321	1.071	1.175
J-765	210.9	0.226	0.321	1.071	1.175

		Existing Futu			ture, Incl. Infill	
		Maximum Peak Hour		Maximum	Peak Hour	
Model	Elevation	Day Demand	Demand	Day Demand	Demand	
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)	
J-770	210.4	0.226	0.321	3.670	3.774	
J-775	207.9	0.226	0.321	0.246	0.350	
J-780	209.5	0.226	0.321	0.246	0.350	
J-785	209.3	0.226	0.321	0.246	0.350	
J-790	210.1	0.226	0.321	0.246	0.350	
J-795	209.4	0.226	0.321	1.071	1.175	
J-800	210.5	0.226	0.321	1.071	1.175	
J-805	211.4	0.226	0.321	1.071	1.175	
J-810	176.7	0.226	0.321	0.246	0.350	
J-815	181.7	0.226	0.321	0.246	0.350	
J-820	180.2	0.226	0.321	0.246	0.350	
J-825	190.1	0.226	0.321	0.246	0.350	
J-830	190.1	0.226	0.321	0.246	0.350	
J-835	197.1		0.321			
J-840	197.0	0.226		0.246	0.350	
		0.226	0.321 0.321	0.246	0.350	
J-845	191.3	0.226 0.226		0.246	0.350	
J-846 J-850	187.3 197.2		0.321	0.246	0.350	
J-855	197.2	0.226	0.321	0.246	0.350	
		0.226	0.321	0.246	0.350	
J-860	200.6	0.226	0.321	0.246	0.350	
J-865	196.8	0.226	0.321	0.246	0.350	
J-870	201.0	0.226	0.321	0.246	0.350	
J-875	197.2	0.226	0.321	0.246	0.350	
J-880	197.9	0.226	0.321	0.246	0.350	
J-885	198.9	0.226	0.321	0.246	0.350	
J-890	201.4	0.226	0.321	0.246	0.350	
J-895	201.3	0.226	0.321	0.246	0.350	
J-900	194.6	0.226	0.321	0.246	0.350	
J-905	181.9	0.226	0.321	0.246	0.350	
J-910	183.1	0.226	0.321	0.246	0.350	
J-915	180.7	0.226	0.321	0.246	0.350	
J-920	182.9	0.226	0.321	0.246	0.350	
J-925	192.6	0.226	0.321	0.246	0.350	
J-930	193.7	0.226	0.321	0.246	0.350	
J-935	182.5	0.226	0.321	0.246	0.350	
J-940	186.5	0.226	0.321	0.246	0.350	
J-945	190.1	0.226	0.321	0.246	0.350	
J-950	197.6	0.226	0.321	0.246	0.350	
J-955	197.8	0.226	0.321	0.246	0.350	
J-960	192.8	0.226	0.321	0.246	0.350	
J-965	192.0	0.226	0.321	0.246	0.350	
J-970	191.9	0.226	0.321	0.246	0.350	
J-975	199.0	0.226	0.321	0.246	0.350	
J-980	198.9	0.226	0.321	0.246	0.350	
J-985	188.8	0.226	0.321	0.246	0.350	
J-990	191.3	0.226	0.321	0.246	0.350	
J-995	179.9	0.226	0.321	0.246	0.350	
J-1000	181.6	0.226	0.321	0.246	0.350	
J-1005	181.8	0.226	0.321	0.246	0.350	
J-1010	184.3	0.226	0.321	0.246	0.350	
J-1015	184.0	0.226	0.321	0.246	0.350	

		Exis	ting	Future, I	Future, Incl. Infill		
		Maximum	Peak Hour	Maximum	Peak Hour		
Model	Elevation	Day Demand	Demand	Day Demand	Demand		
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)		
J-1020	190.4	0.226	0.321	0.246	0.350		
J-1025	191.0	0.226	0.321	0.246	0.350		
J-1030	192.0	0.226	0.321	0.246	0.350		
J-1035	192.3	0.226	0.321	0.246	0.350		
J-1040	190.5	0.226	0.321	0.246	0.350		
J-1045	192.1	0.226	0.321	0.246	0.350		
J-1050	192.1	0.226	0.321	0.246	0.350		
J-1055	194.6	0.226	0.321	0.246	0.350		
J-1060	195.0	0.226	0.321	0.246	0.350		
J-1065	181.6	0.226	0.321	0.246	0.350		
J-1070	189.1	0.226	0.321	0.246	0.350		
J-1075	189.0	0.226	0.321	0.246	0.350		
J-1080	191.3	0.226	0.321	0.246	0.350		
J-1085	194.0	0.226	0.321	0.246	0.350		
J-1083	194.0	0.226	0.321	0.246	0.350		
J-1095	198.2	0.226	0.321	0.246	0.350		
J-1110	196.2	0.226	0.321	0.246	0.350		
J-1115	196.2	0.226	0.321	0.246	0.350		
J-1115	202.1	0.226	0.321	0.246	0.350		
J-1125	205.3						
		0.226	0.321	0.246	0.350		
J-1135	200.8	0.226	0.321	0.246	0.350		
J-1140	200.5	0.226	0.321	0.246	0.350		
J-1145	198.7	0.226	0.321	0.246	0.350		
J-1160	198.6	0.226	0.321	0.246	0.350		
J-1165	200.8	0.226	0.321	0.246	0.350		
J-1170	199.9	0.226	0.321	0.246	0.350		
J-1175	205.9	0.226	0.321	0.246	0.350		
J-1185	202.6	0.226	0.321	0.246	0.350		
J-1195	203.8	0.226	0.321	0.246	0.350		
J-1205	201.7	0.226	0.321	0.246	0.350		
J-1210	206.0	0.226	0.321	0.246	0.350		
J-1215	205.5	0.226	0.321	0.246	0.350		
J-1220	208.1	0.226	0.321	5.224	5.328		
J-1225	206.7	0.226	0.321	0.246	0.350		
J-1230	208.5	0.226	0.321	0.246	0.350		
J-1235	207.1	0.226	0.321	0.246	0.350		
J-1240	210.6	0.226	0.321	0.246	0.350		
J-1245	210.9	0.226	0.321	0.246	0.350		
J-1250	211.1	0.226	0.321	0.246	0.350		
J-1255	209.5	0.226	0.321	0.246	0.350		
J-1260	212.7	0.226	0.321	0.246	0.350		
J-1265	213.3	0.226	0.321	0.246	0.350		
J-1275	214.3	0.226	0.321	0.246	0.350		
J-1280	215.0	0.226	0.321	0.246	0.350		
J-1285	198.8	0.226	0.321	0.246	0.350		
J-1290	199.4	0.226	0.321	0.246	0.350		
J-1295	202.1	0.226	0.321	0.246	0.350		
J-1300	202.9	0.226	0.321	0.246	0.350		
J-1305	199.0	0.226	0.321	0.484	0.588		
J-1310	199.8	0.226	0.321	0.246	0.350		

		Existing		Future, Incl. Infill		
		Maximum	Peak Hour	Maximum	Peak Hour	
Model	Elevation	Day Demand	Demand	Day Demand	Demand	
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)	
J-1320	210.0	0.226	0.321	0.246	0.350	
J-1325	211.9	0.226	0.321	0.246	0.350	
J-1330	202.9	0.226	0.321	0.975	1.078	
J-1335	210.9	0.226	0.321	0.959	1.063	
J-1340	210.7	0.226	0.321	0.246	0.350	
J-1345	210.0	0.226	0.321	5.224	5.328	
J-1350	190.8	0.226	0.321	0.246	0.350	
J-1355	195.3	0.226	0.321	0.246	0.350	
J-1360	201.7	0.226	0.321	5.224	5.328	
J-1365	179.0	0.226	0.321	0.246	0.350	
J-1370	181.8	0.226	0.321	0.246	0.350	
J-1375	187.2	0.226	0.321	0.246	0.350	
J-1380	189.4	0.226	0.321	0.246	0.350	
J-1385	189.8	0.226	0.321	0.246	0.350	
J-1390	189.9	0.226	0.321	0.246	0.350	
J-1395	191.3	0.226	0.321	0.246	0.350	
J-1400	190.4	0.226	0.321	0.246	0.350	
J-1405	188.5	0.226	0.321	0.246	0.350	
J-1410	191.7	0.226	0.321	0.246	0.350	
J-1415	195.4	0.226	0.321	0.246	0.350	
J-1410	195.4	0.226	0.321	0.246	0.350	
J-1425	193.3	0.226	0.321	0.246	0.350	
J-1425	191.7	0.226	0.321	0.246	0.350	
J-1435	191.1	0.226	0.321	0.246	0.350	
J-1440	195.5	0.226	0.321	0.246	0.350	
J-1445	184.0	0.226	0.321	0.246	0.350	
J-1445	192.2	0.226	0.321	1.329	1.433	
J-1450	192.2					
J-1451 J-1455	190.4	0.226 0.226	0.321 0.321	1.329	1.433 1.433	
J-1460	190.4			1.329		
J-1460 J-1461		0.226	0.321 0.321	1.329	1.433	
	198.6	0.226		1.329	1.433	
J-1465	185.6	0.226	0.321	0.246	0.350	
J-1470	189.5	0.226	0.321	0.246	0.350	
J-1475	191.2	0.226	0.321	1.329	1.433	
J-1480	202.4	0.226	0.321	1.329	1.433	
J-1485	201.6	0.226	0.321	0.246	0.350	
J-1490	203.3	0.226	0.321	0.246	0.350	
J-1495	201.8	0.226	0.321	0.246	0.350	
J-1500	199.5	0.226	0.321	0.246	0.350	
J-1505	202.4	0.226	0.321	0.246	0.350	
J-1510	189.5	0.226	0.321	0.246	0.350	
J-1515	190.5	0.226	0.321	0.246	0.350	
J-1520	196.9	0.226	0.321	0.246	0.350	
J-1525	195.1	0.226	0.321	0.246	0.350	
J-1530	185.9	0.226	0.321	0.246	0.350	
J-1535	190.9	0.226	0.321	0.246	0.350	
J-1540	191.7	0.226	0.321	0.246	0.350	
J-1545	193.1	0.226	0.321	0.246	0.350	
J-1550	196.3	0.226	0.321	0.246	0.350	
J-1555	198.5	0.226	0.321	0.246	0.350	
J-1560	188.9	0.226	0.321	0.246	0.350	

		Exis	ting	Future, Incl. Infill		
		Maximum	Peak Hour	Maximum	Peak Hour	
Model	Elevation	Day Demand	Demand	Day Demand	Demand	
Junction	(mASL)	(L/s)	(L/s)	(L/s)	(L/s)	
J-1565	190.2	0.226	0.321	0.246	0.350	
J-1570	198.7	0.226	0.321	0.246	0.350	
J-1575	182.4	0.226	0.321	0.246	0.350	
J-1580	181.7	0.226	0.321	0.246	0.350	
J-1585	184.1	0.226	0.321	0.246	0.350	
J-1590	201.8	0.226	0.321	0.246	0.350	
J-1595	186.6	0.226	0.321	0.246	0.350	
J-1600	199.9	0.226	0.321	0.246	0.350	
J-1605	183.0	0.226	0.321	0.246	0.350	
J-1610	189.9	0.226	0.321	0.246	0.350	
J-1615	183.4	0.226	0.321	0.246	0.350	
J-1620	183.3	0.226	0.321	0.246	0.350	
J-1625	186.6	0.226	0.321	0.246	0.350	
J-1630	186.7	0.226	0.321	0.246	0.350	
J-1635	180.0	0.226	0.321	0.246	0.350	
J-1640	183.7	0.226	0.321	0.246	0.350	
J-1645	181.6	0.226	0.321	0.246	0.350	
J-1650	182.3	0.226	0.321	0.246	0.350	
J-1655	182.0	0.226	0.321	0.246	0.350	
J-1660	185.8	0.226	0.321	0.246	0.350	
J-1665	185.0	0.226	0.321	0.246	0.350	
J-1670	182.0	0.226	0.321	0.246	0.350	
J-1675	180.5	0.226	0.321	0.246	0.350	
J-1680	180.5	0.226	0.321	0.246	0.350	
J-1685	186.6	0.226	0.321	0.246	0.350	
J-1690	184.4	0.226	0.321	1.434	1.538	
J-1695	185.2	0.226	0.321	0.246	0.350	
J-1700	181.6	0.226	0.321	0.246	0.350	
J-1705	183.9	0.226	0.321	0.246	0.350	
J-1710	184.2	0.226	0.321	0.246	0.350	
J-1715	187.2	0.226	0.321	0.246	0.350	
J-1720	187.0	0.226	0.321	0.246	0.350	
J-1725	189.9	0.226	0.321	0.246	0.350	
J-1730	195.6	0.226	0.321	0.246	0.350	
J-1735	189.3	0.226	0.321	0.246	0.350	
J-1740	188.5	0.226	0.321	6.033	6.137	
J-3000	214.1	0.226	0.321	0.246	0.350	
J-3005	212.5	0.226	0.321	0.246	0.350	
J-3010	209.7	0.226	0.321	0.246	0.350	
J-3015	208.9	0.226	0.321	0.888	0.991	
J-3020	205.6	0.226	0.321	0.246	0.350	
J-3025	206.1	0.226	0.321	0.246	0.350	
J-3030	205.1	0.226	0.321	0.246	0.350	
J-3035	203.4	0.226	0.321	0.246	0.350	
J-3040	200.7	0.226	0.321	0.246	0.350	
J-3045	201.8	0.226	0.321	0.246	0.350	
J-3050	202.1	0.226	0.321	0.246	0.350	
J-3055	199.5	0.226	0.321	0.246	0.350	
J-3060	198.5	0.226	0.321	0.246	0.350	
J-3065	198.8	0.226	0.321	0.246	0.350	
Minimum	176.7	0.226	0.321	0.246	0.350	
Maximum	215.0	0.226	0.321	8.064	8.168	
Total		80.5	114.4	149.5	186.4	

3.1 <u>Existing Conditions</u>

For peak hour analysis, assume no pumps operating and standpipe water level at nominal operating level of 248.0 mASL. For fire flow analysis, use three scenarios: standpipe nominal water level of 248.0 mASL with no HLPs, standpipe low water level of 247.0 mASL with HLP3 on, and standpipe low water level of 247.0 mASL with both HLP1 and HLP3 on.

Available fire flow is below 50 L/s at various areas, specifically:

- At J-5, the southwesternmost junction in the model, on Goderich St.
- At J-435, at the dead-end of a cul-de-sac on Hunter St.
- At J-790, at the dead end of a cul-de-sac on Goldie Cr.
- Along Rowan Ave, Edgemere Ave, and Centre Ave, and along Concession Rd 7 west of Hwy. 23 (in the vicinity of Stoney Island Conservation Area)
- Much of Inverhuron

		Available Fire Flow at 140 kPa (L/s)				
Junction	P at Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3 On		
J-5	647	45	45	47		
J-10	548	89	93	97		
J-15	533	90	95	99		
J-20	493	83	87	91		
J-25	465	73	76	80		
J-30	499	85	90	94		
J-35	507	83	87	90		
J-40	518	99	105	109		
J-45	501	76	79	82		
J-50	670	59	61	63		
J-55	635	109	114	117		
J-60	522	100	106	111		
J-65	563	100	106	110		
J-70	564	103	110	114		
J-75	567	105	111	116		
J-80	528	101	108	112		
J-85	467	138	149	155		
J-90	491	123	131	136		
J-95	499	93	97	100		
J-100	467	193	214	224		
J-105	513	193	216	227		
J-110	493	128	136	141		
J-115	498	190	212	223		
J-120	484	186	207	217		
J-125	452	76	79	83		
J-130	457	175	193	202		
J-135	468	181	201	211		
J-140	464	133	142	148		
J-145	450	93	98	102		

		Available l	Fire Flow at 14	0 kPa (L/s)
Junction	P at Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3
J-150	450	168	185	On 194
	450	164		188
J-155	444	127	180 136	141
J-160 J-165	482		227	
J-170	471	204 147	159	238 165
J-175	461	104	110	114
J-180	466	104	109	114
J-185	501	147	159	165
	495	118	125	
J-190 J-195	495	168	185	130 192
J-200	480	263	312	337
J-205		185	205	214
J-205 J-210	484 473	185	205	214
J-215	477	184	203	213
J-220	477	184	202	212
J-225	477	233	263	278
J-230	485		311	
		269		333
J-235	453	271	314	338
J-240	451	135	144	150
J-245	630	116	121	124
J-250	529	132	140	145
J-255	539	84	88	91
J-260	475	311	432	495
J-265	535	307	443	500
J-270	530	307	453	500
J-275	509	302	441	500
J-280	445	218	250	266
J-285	476	220	250	264
J-290	477	310 221	440	500 274
J-295	443		258	
J-300	450	67	69	72
J-305	470	222	250	263
J-310	457	289	337	364
J-315	471	293	344	372
J-320	461	92 299	96 457	100 500
J-325	539		457	
J-330	488	307	448	500
J-335 J-340	524 507	305 67	69	500 71
	507		500	
J-345 J-350	456	303 99	105	500 109
J-350 J-355	439	192	217	229
J-355 J-360	439	319	391	433
J-365	508	203	225	237
J-365 J-370	508	203 71	74	76
J-370 J-375	487	71	75	
J-380	467	184	202	77 211
J-385	490	66	68	70
		90	94	
J-390 J-395	493 493			97
		86	89	92
J-400	499	77	80	83

		Available	Fire Flow at 14	0 kPa (L/s)
Junction	P at Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3
J-405	451	102	107	On 111
J-410	485	143	153	158
J-415	530	73	75	78
J-420	535	66	68	70
J-425	493	68	71	73
J-430	461	177	194	202
J-435	497	44	44	45
J-440	498	343	415	452
J-445	517	196	212	221
J-450	529	206	222	230
J-455	502	85	88	90
J-460	489	92	96	98
J-465	487	369	437	478
J-470	524	201	218	227
J-475	504	218	238	250
J-480	488	248	276	291
J-485	475	338	420	469
J-490	435	306	432	500
J-495	559	364	459	500
J-500	490	381	452	495
J-505	489	108	112	115
J-510	488	130	136	140
J-515	489	101	104	106
J-520	473	90	93	94
J-525	482	400	500	500
J-530	477	418	500	500
J-535	503	381	487	500
J-540	494	114	119	121
J-545	479	394	500	500
J-550	484	190	204	211
J-555	469	165	176	180
J-560	482	98	101	103
J-565	471	75	78	80
J-570	471	73	75	77
J-575	464	69	70	72
J-580	479	407	500	500
J-585	482	407	500	500
J-590	471	415	500	500
J-595	482	243	262	273
J-600	481	226	243	253
J-605	436	165	175	179
J-610	446	101	105	107
J-615	440	115	120	122
J-620	463	422	500	500
J-625	403	130	137	140
	418		443	487
J-630		376		
J-635	407	133	139	142
J-640	367	88	91	93
J-645	466	95	98	99
J-650	422	100	102	103
J-655	413	130	134	136

		Available	Fire Flow at 14	· · · · ·
Junction	P at Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3 On
J-660	408	157	162	165
J-665	406	153	158	160
J-670	445	142	146	147
J-675	406	194	203	208
J-680	412	114	116	118
J-685	405	100	101	102
J-690	403	180	186	190
J-695	401	179	184	187
J-700	409	440	500	500
J-705	407	456	500	500
J-710 J-715	397	500 500	500	500
	396		500	500
J-720	400	465	500	500
J-725	380	142	146	148
J-730	369	98	100	101
J-735	406	205	222	230
J-740	394	355	431	477
J-745	388	158	168	173
J-750	392	409	500	500
J-755	383	330	395	433
J-760	334	253	289	307
J-765	353	104	109	111
J-770	358	269	307	327
J-775	384	143	148	150
J-780	369	101	103	104
J-785	371	71	72	73
J-790	363	49	49	50
J-795	367	349	421	459
J-800	356	307	360	390
J-805	347	249	280	296
J-810	679	112	118	121
J-815	631	94	98	101
J-820	645	295	493	500
J-825	549	291	484	500
J-830	480	302	500	500
J-835	481	297	494	500
J-840	480	296	492	500
J-845	537	288	476	500
J-846	576	286	474	500
J-850	479	288	478	500
J-855	479	293	485	500
J-860	446	197	229	243
J-865	485	144	156	160
J-870	442	232	278	302
J-875	481	194	217	228
J-880	472	289	475	500
J-885	463	290	477	500
J-890	438	294	485	500
J-895	439	301	465	500
J-900	507	332	500	500
J-905	628	285	475	500

		Available l	Fire Flow at 14	0 kPa (L/s)
	P at Q _{PEAK-HOUR}	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3
Junction	(kPa)			On
J-910	617	285	473	500
J-915	641	285	477	500
J-920	618	283	467	500
J-925	524	287	472	500
J-930	513	287	472	500
J-935	622	281	456	500
J-940	584	281	454	500
J-945	548	281	453	500
J-950	475	279	439	500
J-955	473	291	462	500
J-960	521	282	455	500
J-965	530	273	402	458
J-970	530	271	395	448
J-975	461	265	403	500
J-980	461	250	365	452
J-985	561	97	102	106
J-990	536	210	242	256
J-995	647	140	151	155
J-1000	631	264	322	348
J-1005	629	258	303	325
J-1010	604	210	237	249
J-1015	607	203	228	239
J-1020	544	209	239	252
J-1025	539	197	224	236
J-1030	530	261	393	457
J-1035	526	256	337	372
J-1040	543	58	60	63
J-1045	528	77	81	84
J-1050	528	71	74	77
J-1055	503	146	160	166
J-1060	499	97	104	108
J-1065	630	235	270	287
J-1070	554	199	242	256
J-1075	555	197	224	235
J-1080	533	195	233	246
J-1085	514	100	104	106
J-1090	498	341	500	500
J-1095	471	223	255	271
J-1110	480	332	481	500
J-1115	491	167	182	188
J-11125	433	135	146	150
J-1130	408	59	60	62
J-1135	445	306	396	451
J-1140	449	160	176	183
J-1145	465	268	331	368
J-1160	466	247	298	325
J-1165	443	170	193	202
J-1100 J-1170	443	215	252	269
		215		269
J-1175	394		260	
J-1185	428	125	135	140
J-1195	417	247	294	319

		Available Fire Flow at 140 kPa (L/s)				
	P at Q _{PEAK-HOUR}	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3		
Junction	(kPa)			On		
J-1205	436	243	287	310		
J-1210	393	206	246	266		
J-1215	397	134	149	157		
J-1220	372	83	89	94		
J-1225	404	500	500	500		
J-1230	372	55	57	59		
J-1235	385	62	64	66		
J-1240	351	258	326	365		
J-1245	348	121	132	137		
J-1250	346	257	317	353		
J-1255	365	257	320	352		
J-1260	331	184	213	227		
J-1265	325	150	168	177		
J-1275	316	235	288	318		
J-1280	309	229	279	308		
J-1285	463	236	336	406		
J-1290	455	220	304	358		
J-1295	430	167	190	200		
J-1300	423	112	122	127		
J-1305	458	184	240	269		
J-1310	450	182	236	264		
J-1315	376	76	81	87		
J-1320	350	114	132	140		
J-1325	332	96	108	116		
J-1330	419	188	246	276		
J-1335	342	86	95	101		
J-1340	344	93	104	111		
J-1345	351	72	78	84		
J-1350	537	158	174	181		
J-1355	493	113	122	127		
J-1360	430	185	222	239		
J-1365	654	206	228	238		
J-1370	625	87	91	94		
J-1375	571	94	99	102		
J-1380	551	190	214	224		
J-1385	547	190	214	225		
J-1390	546	193	224	236		
J-1395	532	192	223	235		
J-1400	540	147	161	166		
J-1405	558	119	128	132		
J-1410	527	138	151	156		
J-1415	492	190	221	234		
J-1420	492	120	131	136		
J-1425	528	127	137	142		
J-1430	532	147	161	167		
J-1435	518	73	76	80		
J-1440	491	71	74	78		
J-1445	602	73	75	78		
J-1450	521	158	185	193		
J-1451	527	155	180	190		
J-1455	539	149	172	181		
				_		

		Available Fire Flow at 140 kPa (L/s)				
	P at Q _{PEAK-HOUR}	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3		
Junction	(kPa)			On		
J-1460	470	148	170	179		
J-1461	459	155	180	191		
J-1465	586	61	63	66		
J-1470	546	135	153	161		
J-1475	531	138	158	166		
J-1480	421	136	155	163		
J-1485	428	133	150	158		
J-1490	411	129	146	153		
J-1495	425	122	136	143		
J-1500	448	119	133	139		
J-1505	419	112	124	131		
J-1510	544	63	65	68		
J-1515	534	65	67	71		
J-1520	472	104	114	120		
J-1525	489	100	110	115		
J-1530	578	39	40	42		
J-1535	530	41	42	45		
J-1540	521	57	59	62		
J-1545	508	83	88	92		
J-1550	477	94	102	108		
J-1555	455	91	98	104		
J-1560	548	47	49	51		
J-1565	536	53	55	58		
J-1570	452	84	90	95		
J-1575	610	36	36	38		
J-1580	617	39	40	42		
J-1585	594	39	40	43		
J-1590	421	78	83	88		
J-1595	568	75	80	85		
J-1600	439	75	80	85		
J-1605	604	75	80	85		
J-1610	536	64	66	70		
J-1615	599	73	77	82		
J-1620	600	73	77	81		
J-1625	568	73	77	81		
J-1630	567	73	77	81		
J-1635	632	72	75	80		
J-1640	596	69	72	76		
J-1645	617	69	72	77		
J-1650	610	69	72	77		
J-1655	612	56	57	60		
J-1660	575	56	57	60		
J-1665	583	67	70	75		
J-1670	612	63	66	70		
J-1675	626	59	62	66		
J-1680	626	58	60	64		
J-1685	567	61	64	68		
J-1690	588	55	57	61		
J-1695	580	53	55	59		
J-1700	615	49	51	53		
J-1705	593	52	54	58		
L			1	İ		

		Available Fire Flow at 140 kPa (L/s)			
Junction	P at Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & HLP3 On	
J-1710	590	45	47	49	
J-1715	560	52	54	58	
J-1720	563	51	53	57	
J-1725	535	50	52	56	
J-1730	479	41	42	45	
J-1735	540	48	50	54	
J-1740	547	49	51	55	
J-3000	317	238	293	325	
J-3005	333	244	304	339	
J-3010	360	251	315	354	
J-3015	367	258	327	369	
J-3020	399	270	348	397	
J-3025	395	282	370	428	
J-3030	405	292	389	455	
J-3035	421	302	409	484	
J-3040	447	311	430	500	
J-3045	437	315	440	500	
J-3050	434	321	454	500	
J-3055	460	325	463	500	
J-3060	469	328	470	500	
J-3065	466	330	477	500	
Min	309	36	36	38	
Max	679	500	500	500	

denotes operating pressure less than 275 kPa
denotes operating pressure above 275 kPa but less than 350 kPa
denotes operating pressure greater than 480 kPa
denotes fire flow of less than 50 L/s at 140 kPa minimum system pressure

4.0 Model Results - 2043 Demands

4.1 <u>Existing Watermain Conditions</u>

Initial trial for 2043 demand conditions is performed with existing watermain installations remaining at existing diameters.

Based on the results summarized below, under future peak hour demand, pressures at all junctions are acceptable. Pressures are lowest near the northeast boundary, specifically in the following areas:

- Along Sutton St. east of Gary St., along North St., and along Knights Ct.
- Along Millennium Way and Durham St. east of Hwy. 21, due to the large demand associated with the 9 & 21 Business Park and 7 Millennium Way, and the dead-ends modelled at J-760, J-765 and J-805

Available fire flow is acceptable at the above locations, but is below 50 L/s at various other areas, specifically:

- At J-5, the southwesternmost junction in the model, on Goderich St.
- At J-435, at the dead-end of a cul-de-sac on Hunter St.
- At J-790, at the dead end of a cul-de-sac on Goldie Cr.
- Along Rowan Ave, Edgemere Ave, and Centre Ave, and along Concession Rd 7 west of Hwy. 23 (in the vicinity of Stoney Island Conservation Area)
- Much of Inverhuron

	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-5	619	43	45	46
J-10	520	83	91	94
J-15	506	84	92	96
J-20	466	78	85	89
J-25	437	69	75	78
J-30	471	80	87	91
J-35	480	79	85	88
J-40	490	92	102	106
J-45	473	71	77	80
J-50	642	57	60	61
J-55	607	104	112	115
J-60	494	93	104	108
J-65	536	93	103	107
J-70	537	96	107	111
J-75	540	97	108	113
J-80	501	94	105	109
J-85	439	127	144	151
J-90	464	114	128	133
J-95	471	87	94	97
J-100	439	171	200	213
J-105	485	169	199	213
J-110	466	119	131	136

	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-115	471	167	196	209
J-120	456	164	191	204
J-125	424	71	76	79
J-130	429	154	179	190
J-135	440	160	186	198
J-140	436	121	135	142
J-145	422	86	94	98
J-150	422	149	172	182
J-155	416	145	167	177
J-160	438	116	129	135
J-165	455	183	212	226
J-170	444	134	151	158
J-175	433	96	105	109
J-180	438	96	105	109
J-185	473	137	154	161
J-190	467	110	121	126
J-195	467	155	178	188
J-200	453	235	294	325
J-205	456	170	196	207
J-210	446	168	195	206
J-215	450	169	194	205
J-220	451	169	193	204
J-225	453	211	248	267
J-230	459	239	289	316
J-235	427	241	292	320
J-240	426	125	139	146
J-245	602	110	119	122
J-250	502	123	137	142
J-255	512	80	86	89
J-260	448	240	397	470
J-265	507	237	414	480
J-270	503	237	420	493
J-275	482	234	412	484
J-280	418	196	237	257
J-285	449	199	237	255
J-290	450	239	403	480
J-295	417	199	244	265
J-300	423			71
J-305	444	63 202	68 237	254
J-310	432	202	314	254 346
J-315	432		314	
		260		353
J-320	435	86	94	97
J-325	512	232	428	500
J-330	461	237	416	492
J-335	497	236	442	500
J-340	484	65	69	70
J-345	481	234	437	500
J-350	432	94	103	107
J-355	415	176	209	224
J-360	415	267	368	417
J-365	482	183	212	226
J-370	477	68	72	74
J-375	461	68	73	75

	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-380	446	166	190	202
J-385	464	62	66	68
J-390	467	84	91	94
J-395	467	81	87	89
J-400	473	73	78	80
J-405	427	95	104	108
J-410	460	134	148	154
J-415	504	70	74	76
J-420	509	63	67	68
J-425	467	65	69	71
J-430	435	160	183	193
J-435	479	42	44	44
J-440	476	294	383	432
J-445	495	184	206	217
J-450	508	193	215	226
J-455	480	82	87	89
J-460	468	88	94	97
J-465	466	302	410	456
J-470	503	187	211	223
J-475	482	203	231	244
J-480	467	230	266	284
J-485	453	272	393	450
J-490	409	237	400	481
J-495	537	280	428	496
J-500	469	305	424	472
J-505	468	102	110	113
J-510	468	123	133	138
J-515	471	97	103	105
J-520	455	87	91	93
J-525	463	313	500	500
J-530	458	326		500
J-535	482	295	457	500
J-540	473	108	117	120
J-545	460	307	500	500
J-550	464	178	198	208
J-555	450	156	171	179
J-560	463	93	99	101
J-565	450	72	77	78
J-570	450	70	74	76
J-575	443	66	70	71
J-580	460	318	500	500
J-585	463	318	500	500
J-590	452	325	500	500
J-595	464	229	254	268
J-600	463	212	236	249
J-605	418	155	171	178
J-610	428	97	103	105
J-615	410	109	118	121
J-620	444	329	497	500
J-625	400	123	134	138
J-630	399	337	415	464
J-635	390	124	134	138
J-640	350	82	88	90

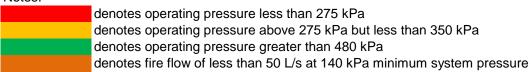
	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-645	448	91	96	98
J-650	408	96	100	102
J-655	399	125	132	135
J-660	395	151	160	164
J-665	394	147	155	159
J-670	432	137	144	147
J-675	394	185	199	205
J-680	399	110	115	117
J-685	395	97	100	101
J-690	391	173	183	188
J-695	390	172	181	186
J-700	393	352	500	500
J-705	392	370	500	500
J-710	390	500	500	500
J-715	386	452	500	500
J-720	387	383	500	500
J-725	369	136	144	147
J-730	357	94	98	100
J-735	390	185	210	222
J-740	377	306	393	447
J-745	371	145	161	167
J-750	377	335	465	500
J-755	366	286	362	408
J-760	317	225	270	294
J-765	335	97	105	108
J-770	341	238	286	311
J-775	373	137	145	149
J-780	357	97	101	103
J-785	360	69	71	72
J-790	352	48	49	50
J-795	351	289		439
J-800	339	258	336	374
J-805	330	222	262	283
J-810	652	108	117	120
J-815	603	90	97	100
J-810	618	229	423	500
J-825	521	229	423	500
J-830	453	234	416	500
J-835	453	234	435	500
J-835 J-840	454	230	424	500
J-845	510		422	500
J-845 J-846	549	223 221	410	500
J-846 J-850	452	221	408	500
J-855	452			
J-855 J-860	432	228	417	500
	461	179	219 152	239
J-865		135		159
J-870	416	209	264	294
J-875	457	180	210	225
J-880	445	225	408	500
J-885	436	226	409	500
J-890	411	228	416	500
J-895	413	234	426	500
J-900	484	258	459	500

	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-905	601	220	406	500
J-910	590	221	406	500
J-915	613	220	408	500
J-920	591	218	396	500
J-925	497	222	406	500
J-930	486	223	406	500
J-935	595	215	384	500
J-940	556	216	384	500
J-945	521	216	383	500
J-950	448	217	374	500
J-955	446	226	396	500
J-960	494	218		500
J-965	501	210	360	437
J-970	502	208	355	427
J-975	433	205	342	453
J-980	432	193	310	398
J-985	531	92	100	104
J-990	507	193	231	251
J-995	618	133	147	152
J-1000	601	197	306	337
J-1005	599	192	287	314
J-1010	574	192	227	243
J-1015	576	187	218	233
J-1020	515	191	228	246
J-1025	509	180	214	230
J-1030	500	199	331	432
J-1035	497	195	314	354
J-1040	513	55	59	61
J-1045	498	73	79	82
J-1050	498	67	72	75
J-1055	472	133	153	162
J-1060	468	91	101	105
J-1065	600	185	256	278
J-1070	516	148	212	237
J-1075	516	147	203	218
J-1080	495	146	209	227
J-1085	491	95	103	105
J-1090	476	266	463	500
J-1095	446	205	244	265
J-1110	457	260	426	500
J-1115	469	156	177	186
J-1125	411	125	142	149
J-1130	392	57	60	61
J-1135	419	236	368	429
J-1140	425	148	170	181
J-1145	437	217	309	352
J-1160	438	212	279	312
J-1165	413	151	181	195
J-1170	425	192	237	260
J-1175	366	187	240	269
J-1185	405	116	132	138
J-1195	393	221	276	307
J-1205	412	218	270	299

	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-1210	365	179	228	253
J-1215	367	116	139	148
J-1220	341	71	81	86
J-1225	404	500	500	500
J-1230	350	52	56	57
J-1235	363	58	63	65
J-1240	329	219	299	347
J-1245	327	111	127	134
J-1250	324	219	294	337
J-1255	346	217	294	339
J-1260	310	162	201	221
J-1265	303	133	161	172
J-1205	295	202	266	304
	289			
J-1280		197	259	295
J-1285	432	183	285	357
J-1290	423	169	257	314
J-1295	399	139	176	191
J-1300	392	100	115	121
J-1305	422	139	198	231
J-1310	414	137	195	227
J-1315	338	62	72	76
J-1320	313	90	114	123
J-1325	295	78	96	103
J-1330	381	141	203	237
J-1335	305	69	83	89
J-1340	306	75	90	97
J-1345	312	58	68	73
J-1350	498	139	160	169
J-1355	453	101	114	119
J-1360	390	142	187	207
J-1365	618	160	215	227
J-1370	585	81	87	90
J-1375	530	85	93	96
J-1380	511	146	195	208
J-1385	507	145	194	208
J-1390	505	145	193	216
J-1395	491	144	193	215
J-1400	499	129	147	154
J-1405	517	107	119	124
J-1410	485	121	138	145
J-1415	452	143	191	211
J-1420	452	106	121	126
J-1425	488	112	127	133
J-1430	490	127	145	153
J-1435	478	67	72	75
J-1440	451	64	70	73
J-1445	559	67	72	74
J-1450	478	123	154	168
J-1451	484	121	150	164
J-1455	495	116	143	155
J-1460	426	116	143	155
J-1461	416	122	152	165
J-1465	542	56	60	62

	P at	Available Fire Flow at 140 kPa (L/s)		
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On
J-1470	501	106	129	138
J-1475	486	109	133	143
J-1480	376	107	130	140
J-1485	383	105	127	136
J-1490	366	102	123	132
J-1495	379	97	115	123
J-1500	401	95	112	120
J-1505	372	89	105	112
J-1510	496	56	60	63
J-1515	485	57	62	64
J-1520	424	82	96	102
J-1525	440	79	92	98
J-1530	527	36	38	39
J-1535	481	37	40	41
J-1540	470	50	54	57
J-1545	457	71	78	82
J-1550	427	74	86	91
J-1555	404	71	83	88
J-1560	498	42	45	47
J-1565	486	47	51	53
J-1570	400	66	76	80
J-1575	557	33	34	35
J-1580	564	36	38	39
J-1585	541	36	38	39
J-1590	368	61	70	74
J-1595	513	60	67	71
J-1600	385	60	67	71
J-1605	548	60	67	71
J-1610	479	54	59	61
J-1615	543	58	65	68
J-1620	544	58		68
J-1625	512	58	65	68
J-1630	510	58	65	68
J-1635	576	57	63	66
J-1640	539	54	60	64
J-1645	559	55	61	64
J-1650	552	55	61	64
J-1655	555	49	52	54
J-1660	517	48	51	53
J-1665	525	53	59	62
J-1670	553	50	55	58
J-1675	566	47	51	54
J-1680	565	45	50	52
J-1685	508	48	53	56
J-1690	526	43	47	50
J-1695	517	41	45	48
J-1700	552	41	45	46
J-1705	530	41	45	47
J-1705	527	39	45	43
J-1710 J-1715	497	40	41	43
J-1715	497	40	43	46
J-1725	499	39	43	46
J-1730	415	33	35	37

	P at	Available Fire Flow at 140 kPa (L/s)			
Junction	Q _{PEAK-HOUR} (kPa)	Standpipe 248.0 mASL, HLPs Off	Standpipe 247.0 mASL, HLP3 On	Standpipe 247.0 mASL, HLP1 & 3 On	
J-1735	476	38	41	44	
J-1740	484	38	42	44	
J-3000	296	204	271	310	
J-3005	311	209	280	323	
J-3010	338	214	289	335	
J-3015	345	219	299	349	
J-3020	377	228	317	374	
J-3025	372	237	336	402	
J-3030	382	245	352	426	
J-3035	398	251	368	451	
J-3040	425	257	384	478	
J-3045	414	257	393	491	
J-3050	411	258	404	500	
J-3055	437	259	411	500	
J-3060	447	259	417	500	
J-3065	444	260	423	500	
Min	289	33	34	35	
Max	652	500	500	500	



Appendix D Tiverton WaterCAD® Modelling Information

Municipality of Kincardine
WaterCAD Modelling for Master Plan
Calculations and Notes for Tiverton

Job #: 22128

Date: November 21, 2022

Revised:

1.0 Background

The Municipality of Kincardine is completing a water and wastewater Master Plan update process. The water supply component will include a review of servicing existing development and future development for peak hour and maximum day + fire flow demands. The purpose of these notes is to summarize data used to update the WaterCAD model, and the results of that modelling for the community of Tiverton. The model was originally created as part of BMROSS project 16130.

2.0 Analysis & Model Data

2.1 Data

Reference	<u>Item</u>			
22128	Existing avg. day demand		2.6	L/s
		=	222	m ³ /d
	Existing max. day demand		7.1	L/s
		=	616	m ³ /d
22128	Tiverton town pop. (2021)		717	persons
22128	Min Tiverton town pop. (20-	43)		persons
22128	Max Tiverton town pop. (20	143)	942	persons
MECP	Peak hour factor - ex. pop.		4.13	
MECP	Peak hour factor - fut. pop.		4.13	
	Ex. peak hour estimate		10.60	L/s
DWWP	Dent Well No. 2 Pump ratir	ng	4.6	L/s
		@	50.6	m TDH
	Briar Hill Well No. 1 Pump		6.1	L/s
		@		m TDH
	Briar Hill Well No. 2 Pump			L/s
		@	50.6	m TDH
Town info	Pumps off (tower level)			mASL
	Pumps on (tower level)		278.2	mASL
DWWP/	Standpipe			
78071	Total volume		1500	m^3
	Usable volume		350	m^3
	Diameter		8	m
	HWL		279.20	mASL
	Grade at base		244.30	mASL

	Standpipe BPS			
	Pump rating			18 L/s
DWWP/		@		13 m TDH
78071/				38 L/s
19036		@	•	13.5 m TDH
19030				67 L/s
		@	•	19.8 m TDH
	Floor elevation			mASL
MOE Guide	Pipe C-factors Pipe Dia. (mm) 150 200-250 300-600 >600		<u>C</u> 100 110 120 130	

MOE Guide	Normal operating pressure range target	350 to 480 kPa
MOE Guide	Normal operating pressure minimum	275 kPa
MOE Guide	Fire flow system pressure minimum	140 kPa
MOE Guide	Maximum allowable system pressure	700 kPa

2.2 Water Demands by Junction

(a) Existing Conditions

Number of junctions - existing model	57
Average day demand per junction	0.045 L/s
Max. day demand per junction	0.125 L/s
Peak hour demand per junction	0.186 L/s

See attached map for area junctions.

Design fire flow demands will vary from about 50 L/s for residential areas to 150 L/s or greater in ICI areas. Considering the relatively small demand associated with consumption as compared to fire flow, and the fact that there are few customers with significant water demand, the total system demand is distributed evenly over all model junctions.

(b) Future Conditions

Demands for existing development are left unchanged, and the incremental future demand for development areas is applied to the nearest model junctions within or adjacent to the development lands.

With reference to 22128 Maximum Day Water Demand Projections Memo (October 28, 2022)

- Development areas are taken from Table 2C
- Demand is applied based on number of ERUs, regardless of type of development.

Average day demand per unit	0.60 m³/ERU/d
=	0.007 L/ERU/s
Maximum day demand per unit	1.66 m ³ /ERU/d
=	0.019 L/ERU/s
Peak hour demand per unit	2.5 m³/ERU/d
=	0.028 L/ERU/s

Development Name	Projected No. of ERUs	Average Day Demand (L/s)	Maximum Day Demand (L/s)	Peak Hour Demand (L/s)	Model Junction to Apply Demand
Conquergood	109	0.75	2.09	3.10	J-5, J-25, J-35, J-60
Pine Tree Campground	59	0.41	1.13	1.68	J-85
Kaydan Drive	14	0.10	0.27	0.40	J-75
Maple Street	14	0.10	0.27	0.40	J-190
Rae Street (Karn Dev.)	25	0.17	0.48	0.71	J-255
Mackwade	5	0.03	0.10	0.14	J-225, J-245
Infill Allowance	30	0.21	0.58	0.85	Distributed

		Exis	sting	Fut	ure
		Maximum		Maximum	
Model	Elevation	Day Demand	Peak Hour	Day Demand	Peak Hour
Junction	(mASL)	(L/s)	Demand (L/s)	(L/s)	Demand (L/s)
J-5	235.0	0.125	0.186	0.659	0.977
J-10	239.0	0.125	0.186	0.135	0.201
J-15	239.0	0.125	0.186	0.135	0.201
J-20	239.0	0.125	0.186	0.135	0.201
J-25	240.0	0.125	0.186	0.659	0.977
J-30	241.0	0.125	0.186	0.135	0.201
J-35	241.0	0.125	0.186	0.659	0.977
J-40	241.0	0.125	0.186	0.135	0.201
J-45	241.0	0.125	0.186	0.135	0.201
J-50	238.0	0.125	0.186	0.135	0.201
J-55	242.0	0.125	0.186	0.135	0.201
J-60	238.0	0.125	0.186	0.659	0.977
J-65	241.0	0.125	0.186	0.135	0.201
J-70	242.0	0.125	0.186	0.135	0.201
J-75	240.0	0.125	0.186	0.404	0.600
J-80	241.0	0.125	0.186	0.135	0.201
J-85	240.0	0.125	0.186	1.269	1.881
J-90	243.0	0.125	0.186	0.135	0.201
J-95	242.0	0.125	0.186	0.135	0.201
J-100	245.0	0.125	0.186	0.135	0.201
J-105	240.0	0.125	0.186	0.135	0.201
J-110	244.0	0.125	0.186	0.135	0.201
J-115	237.0	0.125	0.186	0.135	0.201
J-120	241.0	0.125	0.186	0.135	0.201
J-125	241.0	0.125	0.186	0.135	0.201
J-130	245.0	0.125	0.186	0.135	0.201
J-135	245.0	0.125	0.186	0.135	0.201
J-140	237.0	0.125	0.186	0.135	0.201
J-145	245.0	0.125	0.186	0.135	0.201
J-150	241.0	0.125	0.186	0.135	0.201
J-155	245.0	0.125	0.186	0.135	0.201
J-160	237.0	0.125	0.186	0.135	0.201
J-165	241.0	0.125	0.186	0.135	0.201
J-170	236.0	0.125	0.186	0.135	0.201
J-175 J-180	245.0	0.125	0.186	0.135 0.135	0.201
J-185	239.0 241.0	0.125 0.125	0.186 0.186	0.135	0.201 0.201
J-185 J-190	237.0	0.125	0.186	0.135	0.600
J-195	243.0	0.125	0.186	0.404	0.800
J-200	243.0	0.125	0.186	0.135	0.201
J-205	239.0	0.125	0.186	0.135	0.201
J-210	242.0	0.125	0.186	0.135	0.201
J-215	242.0	0.125	0.186	0.135	0.201
J-220	242.0	0.125	0.186	0.135	0.201
J-225	244.0	0.125	0.186	0.183	0.272
J-230	244.0	0.125	0.186	0.135	0.201
J-235	241.0	0.125	0.186	0.135	0.201
J-240	243.0	0.125	0.186	0.135	0.201
J-245	246.0	0.125	0.186	0.183	0.272
J-250	242.0	0.125	0.186	0.135	0.201
J-255	242.0	0.125	0.186	0.616	0.913
J-260	246.0	0.125	0.186	0.135	0.201
J-265	245.0	0.125	0.186	0.135	0.201

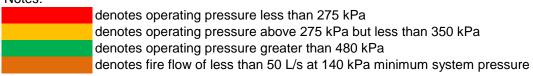
		Existing		Fut	ure
		Maximum		Maximum	
Model	Elevation	Day Demand	Peak Hour	Day Demand	Peak Hour
Junction	(mASL)	(L/s)	Demand (L/s)	(L/s)	Demand (L/s)
J-270	247.0	0.125	0.186	0.135	0.201
J-275	245.0	0.125	0.186	0.135	0.201
J-280	245.0	0.125	0.186	0.135	0.201
J-285	240.0	0.125	0.186	0.135	0.201
Minimum	235.0	0.125	0.186	0.135	0.201
Maximum	247.0	0.125	0.186	1.269	1.881
Total		7.1	10.6	12.0	17.9

3.1 <u>Existing Conditions</u>

For peak hour analysis, assume no pumps operating and standpipe water level at nominal operating level of 278.7 mASL. For fire flow analysis, use two scenarios: standpipe nominal water level of 278.7 mASL with no HLPS, and standpipe low water level of 278.2 mASL with Dent Well 2 and Briar Hill Well 1 on.

		Available Fire Flow at 140 kPa (L/s)			
Junction	P at Q _{PEAK} (kPa)	Standpipe 278.7 mASL, HLPs Off	Standpipe 278.2 mASL, DW2 & BH1 On		
J-5	423	37	47		
J-10	384	37	48		
J-15	384	37	49		
J-20	384	38	49		
J-25	374	36	46		
J-30	364	37	48		
J-35	364	34	45		
J-40	364	37	50		
J-45	364	36	49		
J-50	394	4	4		
J-55	355	38	50		
J-60	394	33	46		
J-65	365	44	57		
J-70	355	63	76		
J-75	375	54	57		
J-80	366	85	94		
J-85	375	53	60		
J-90	346	103	119		
J-95	356	83	93		
J-100	328	106	116		
J-105	375	66	78		
J-110	336	94	106		
J-115	404	52	59		
J-120	366	90	104		
J-125	366	98	112		
J-130	328	134	143		
J-135	328	135	144		
J-140	404	60	71		
J-145	328	10	10		
J-150	366	106	120		
J-155	329	221	221		
J-160	404	57	68		
J-165	366	108	122		
J-170	414	58	68		
J-175	330	221	221		
J-180	385	92	103		
J-185	366	99	112		
J-190	404	53	59		
J-195	347	113	129		
J-200	365	9	9		
J-205	385	52	60		
J-210	356	87	95		
J-215	356	87	95		
J-220	356	82	89		

		Available Fire Flow at 140 kPa (L/s)		
			Standpipe	
		Standpipe	278.2 mASL,	
	P at Q _{PEAK}	278.7 mASL,	DW2 & BH1	
Junction	(kPa)	HLPs Off	On	
J-225	336	59	62	
J-230	336	91	100	
J-235	366	58	61	
J-240	346	8	8	
J-245	317	67	72	
J-250	356	75	81	
J-255	356	77	83	
J-260	317	61	64	
J-265	326	75	81	
J-270	307	54	57	
J-275	326	72	77	
J-280	326	63	67	
J-285	375	49	51	
Min	307	4	4	
Max	423	221	221	



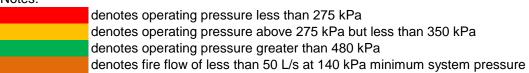
4.0 <u>Model Results - Future Demands</u>

4.1 <u>Existing Conditions</u>

For peak hour analysis, assume no pumps operating and standpipe water level at nominal operating level of 278.7 mASL. For fire flow analysis, use two scenarios: standpipe nominal water level of 278.7 mASL with no HLPS, and standpipe low water level of 278.2 mASL with Dent Well 2 and Briar Hill Well 1 on.

		Available Fire Flow at 140 kPa (L/s)		
Junction	P at Q _{MAX} (kPa)	Standpipe 278.7 mASL, HLPs Off	Standpipe 278.2 mASL, DW2 & BH1 On	
J-5	410	35	45	
J-10	371	35	46	
J-15	371	35	46	
J-20	371	35	46	
J-25	361	33	44	
J-30	352	34	46	
J-35	352	32	43	
J-40	352	35	47	
J-45	352	34	46	
J-50	388	4	4	
J-55	342	36	48	
J-60	381	31	44	
J-65	354	41	54	
J-70	348	60	73	
J-75	369	52	56	
J-80	360	82	92	
J-85	368	51	58	
J-90	340	99	115	
J-95	350	80	90	
J-100	324	103	113	
J-105	368	64	75	
J-110	331	90	103	
J-115	397	50	57	
J-120	359	87	101	
J-125	360	94	109	
J-130	326	131	140	
J-135	326	132	142	
J-140	397	57	68	
J-145	326	10	10	
J-150	360	102	116	
J-155	329	221	221	
J-160	397	55	66	
J-165	360	104	119	
J-170	407	56	66	
J-175	330	221	221	
J-180	379	89	100	
J-185	360	95	108	
J-190	397	51	58	
J-195	342	109	124	
J-200	360	9	9	
J-205	378	50	58	
J-210	350	84	92	

		Available Fire Flow at 140 kPa (L/s)		
Junction	P at Q _{MAX} (kPa)	Standpipe 278.7 mASL, HLPs Off	Standpipe 278.2 mASL, DW2 & BH1 On	
J-215	350	84	92	
J-220	350	79	87	
J-225	331	57	61	
J-230	331	87	97	
J-235	360	56	60	
J-240	340	8	8	
J-245	311	65	70	
J-250	350	73	79	
J-255	350	74	80	
J-260	311	59	63	
J-265	321	72	79	
J-270	301	53	56	
J-275	321	70	75	
J-280	321	61	65	
J-285	369	48	50	
Min	301			
Max	410			



Appendix E Kincardine SewerCAD® Modelling Information

Municipality of Kincardine Kincardine Area SPS Catchments Flow Notes for Master Plan

Job #: 22128

Date: November 16, 2022 Revised: December 13, 2022

1.0 <u>Background</u>

The Municipality of Kincardine is updating its Water & Wastewater Master Plan to evaluate water and wastewater servicing needs for Kincardine, Tiverton, and the Lakeshore Area. The original Master Plan was completed under BMROSS File No. 16130.

The purpose of these notes is to summarize catchment area and design flow information for the Connaught Park, Durham Street, Huron Terrace, Park Street, Goderich Street, and Kincardine Avenue SPSs.

2.0 Design Data

For existing and future flows, the following parameters were used. The "Persons per ERU" parameter was taken from the BMROSS draft memo entitled "Reserve Capacity Analysis for Municipality of Kincardine Major Water and Wastewater Facilities"

The previous Master Plan based residential sewage flows on "units", not Equivalent Residential Units (ERUs). For this Master Plan, existing residential units have been converted to ERUs at a 1:1 ratio (i.e., treating all units as single-family residences, which is conservative), but future developments will be implemented in terms of their actual ERUs, taken from the November 4, 2022 BMROSS memo entitled "Reserve Capacity Analysis for Municipality of Kincardine Major Water and Wastewater Facilities".

To calculate the "Per-ERU average day true sewage flow" (i.e., excluding I-I), inflows to the Kincardine WWTP from 2019 to 2021 were used. The four consecutive months with the lowest inflows in each year were selected as the most representative of "dry-weather" flow conditions. The average day flows from those months were divided by the total number of customers to determine "per household average day true sewage flow".

Reference	<u>Item</u>		
22128 Nov. 4 Reserve Capacity Draft Memo	Persons per ERU Number of customers for Kincardine WW System Existing ERUs from Residential Units in SPS Catchments Average "dry-weather" day flow Per customer average day true sewage flow Per-ERU average day true sewage flow	3,780 3,553 3,170 839	p/ERU customers ERUs m³/day L/cust/day L/ERU/day
22128 Nov. 22 Development Commitments Table	Infill Units - Kincardine-wide	365	ERUs
MECP	I/I allowance		L/ha/s

Guidelines Industrial flow allowance 0.405 L/ha/s
Commercial/Institutional flow allowance 0.324 L/ha/s
22128-Kinc-Water Wastewater Master Plan Undate/Projects/SewerCAD/22128-2022-12-16 SPS c

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3.0 Connaught SPS (BMROSS 16171)

Existing areas and residential units remain unchanged from the previous Master Plan, because no new developments have been constructed in the intervening time. However, given that population densities and true sewage flows have been updated, the existing flow is slightly lower than in the previous Master Plan.

Future flow is based on the existing flow plus flow from the West Ridge on the Lake development (383 ERUs, 15.7 ha), Bradstones (Shepherd) Subdivision (36 ERUs, 1.44 ha), Golf Links Townhouses (7 ERUs, 0.15 ha), and Battler Subdivision (30 ERUs, 1.65 ha), plus a proportionate amount of the total infill ERUs (57.3 ERUs).

Item Residential Area Industrial Area Commercial/Institutional Area ERUs from Residential ERUs as percentage of Kincardine Total	Calc. Ex. 49.81 0 0.54 552 16%	Obs. Ex.	Future 1 68.74 0 0.54 1064	Future 2	Units ha ha ha units
Calculate					
Residential Population	1369		2638		people
Peaking factor	3.71		3.49		-
Average day residential flow	5.36		10.32		L/s
Average day industrial flow	0.00		0.00		L/s
Average day commercial/institutional flow	0.17		0.17		L/s
Average day flow; total	5.53		10.50		L/s
I/I allowance	14.098		19.400		L/s
Allocation for WWTP sludge discharge	3.500		3.500		L/s
Peak instantaneous flow; excl. I/I	20.515		36.636		L/s
Peak instantaneous flow; total incl. I/I	38.113	68.588	59.535	90.010	L/s

4.0 <u>Durham SPS (BMROSS 79017)</u>

Existing areas and residential units remain unchanged from the previous Master Plan, because no new developments have been constructed in the intervening time. However, given that population densities and true sewage flows have been updated, the existing flow is slightly lower than in the previous Master Plan.

Future flow is based on the Durham St. SPS Peak Design flow. The Durham St. SPS upgrade design is underway (BMROSS File No. 18033), and has recently confirmed that the 20-year peak design flow will be 83 L/s. The Durham St. SPS peak design flow was informed by the 17094 Sanitary Design Notes, which accounted for approximately 33 ha of new light industrial development north of Gary St.

Item Residential Area Industrial Area Commercial/Institutional Area ERUs from Residential ERUs as percentage of Kincardine Total	Calc. Ex. 32.42 0 20.53 409 12%	Obs. Ex.	<u>Future</u>	Units ha ha ha units
Calculate Residential Population Peaking factor Average day residential flow Average day industrial flow	1014 3.80 3.97 0.00		Refer to 18033 Durham St. SPS Upgrade Design Notes. SPS 20-Year Peak Design Flow is 83 L/s.	people - L/s L/s
Average day commercial/institutional flow Average day flow; total	6.65 10.6 14.8			L/s L/s L/s
Peak instantaneous flow; excl. I/I Peak instantaneous flow; total incl. I/I	40.3 55.1	39.6	83.0	L/s L/s

5.0 Huron Terrace SPS (BMROSS 79016)

The Huron Terrace SPS service area includes its own catchment area and discharge from the Connaught and Durham SPS's. Calculate catchment area flow and then add Connaught and Durham SPS flows.

Existing areas and residential units remain unchanged from the previous Master Plan, because no new developments have been constructed in the intervening time. The Snobelen Apartment, identified as a development in 16130 Figure C1, was completed at the time of the last Master Plan. However, given that population densities and true sewage flows have been updated, the existing flow is slightly lower than in the previous Master Plan.

In the previous Master Plan, future residential area was calculated on the basis that all development lands north to Concession 5, and between Hwy. 21 and Cty. Rd. 23 would direct sanitary sewage to the Huron Terrace SPS. In this Master Plan, future flows omit lands to the north of the Urban Boundary, and include only the OPF lands (960 ERUs from residential development), 1182 Queen Street (12 ERUs), and 7 Mount Forest Avenue (25 ERUs). The OPF lands also contain approximately 3 ha of proposed institutional area. Infill ERUs (62.1 ERUs) are also included in the future flows proportionally to the catchment area's existing ERUs.

5.1 Huron Terrace Catchment Area Data

<u>Item</u>	Calc. Ex.	Obs. Ex.	Future 1	Future 2	<u>Units</u>
Residential Area	73.18		116.50		ha
Industrial Area	0		0		ha
Commercial/Institutional Area	29.23		32.06		ha
ERUs from Residential	612		1671		units
ERUs as percentage of Kincardine Total	17%				
Calculate					
Residential Population	1518		4145		people
Peaking factor	3.68		3.32		-
Average day residential flow	5.94		16.22		L/s
Average day industrial flow	0.00		0.00		L/s
Average day commercial/institutional flow	9.47		10.39		L/s
Average day flow; total	15.4		26.6		L/s

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I/I allowance	28.7	41.6	L/s
Peak instantaneous flow; total excl. I/I	56.6478	88.3	L/s
Peak instantaneous flow; total incl. I/I	85.3	129.9	L/s

5.2 Huron Terrace SPS Total Flow - Catchment Area + Connaught & Durham SPS Flows

Peak instantaneous flow; total incl. I/I 178.6 274.1 272.5 368.0 L/s

6.0 Park SPS (BMROSS 75056B)

Existing areas and residential units have been updated since the last Master Plan due to the completion of the Penetangore Bluffs (27 ERUs, 3.11 ha), Pipers 1 (1 ERU, 0.12 ha) and Pipers 2 (2 ERUs, 0.18 ha) developments, the completion of the Marriott hotel in the area of the 9&21 Business Park (38 ERUs, 1.1 ha), and the partial completion of the Campbell Avenue development (53 ERUs, 1.68 ha). These numbers are reflected in the "Calculated Ex." column.

An observed peak flow of 115 L/s was observed during a precipitation event in 2013. This value will be used as the existing peak flow for the Park St. SPS for modelling purposes.

Future flow is based on the existing flow plus flow from the Campbell Ave. development (4 ERUs), the Highway 9 & 21 Business Park (in the Jones Consulting plan: 554 ERUs from Residential, 15.29 ha of residential, 3.4 ha of institutional/commercial and remaining from 08055: 28.23 ha of institutional/commercial and 20.8 ha of industrial), 7 Millennium Way (173 ERUs, 0.81 ha of residential), and a proportionate amount of infill ERUs (94.9 ERUs).

Item Residential Area Industrial Area Commercial/Institutional Area ERUs from Residential ERUs as percentage of Kincardine Total	<u>Calc. Ex.</u> 58 0 3.99 950 27%	Obs. Ex.	Future 1 74.42 20.8 35.62 1778	Future 2	Units ha ha ha units
Calculate Residential Population Peaking factor Average day residential flow Average day industrial flow Average day commercial/institutional flow Average day flow; total	2356 3.53 9.22 0.00 1.29 10.51375		4410 3.29508 17.26 8.42 11.54 37.2		people - L/s L/s L/s L/s L/s
I/I allowance	17.4		36.6		L/s
Peak instantaneous flow; excl. I/I Peak instantaneous flow; total incl. I/I	37.1 54.5	96.5	122.7 159.3	201.4	L/s L/s

7.0 Goderich SPS (BMROSS 76007-2)

Existing areas and residential units have been updated since the last Master Plan due to the completion of the Lakefield Phase 1 and Phase 2 developments (39 ERUs, 3.10 ha). Due to the completion of the Lakefield developments, existing flows are slightly higher than in the previous Master Plan.

Future flow is only expected to increase due to infill (56.1 ERUs). No new developments are planned for the Goderich SPS catchment area.

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Item Residential Area Industrial Area Commercial/Institutional Area ERUs from Residential ERUs as percentage of Kincardine Total	<u>Calc. Ex.</u> 62.20 3.89 11.55 540 15.1984%	Obs. Ex.	Future 1 62.20 3.89 11.55 595	Future 2	Units ha ha ha units
Calculate Residential Population Peaking factor Average day residential flow Average day industrial flow Average day commercial/institutional flow Average day flow; total	1339 3.71 5.24 1.58 3.74 10.6		1477 3.68 5.78 1.58 3.74 11.1		people - L/s L/s L/s L/s L/s
I/I allowance	21.7		21.7		L/s
Peak instantaneous flow; excl. I/I Peak instantaneous flow; total incl. I/I	39.2 61.0	29.0	40.9 62.6	30.7	L/s L/s

8.0 Kincardine Avenue SPS (BMROSS 76007-2)

Existing areas and residential units have been updated since the last Master Plan due to the completion of the Stonehaven Phase 1, 2 and 3 developments (48 ERUs, 4.91 ha). Despite the increased number of ERUs, per capita flows have been updated, and the existing flow is still slightly lower than in the previous Master Plan.

Future flow is based on the existing flow plus flow from the Brown Subdivision (245 ERUs), the Brigadoon Subdivision (150 ERUs), the CR Developers project (82 ERUs from hotels and 0.6 ha of commercial land), the 869 Kincardine Ave. trailer park development (88 ERUs), and a proportionate amount of infill ERUs (51.1 ERUs).

Item	Calc. Ex.	Obs. Ex.	Future 1	Future 2	Units
Residential Area	57.48	·	104.51		ha
Industrial Area	0		0		ha
Commercial/Institutional Area	3.99		4.59		ha
ERUs from Residential	490		1105		units
ERUs as percentage of Kincardine Total	14%				
Calculate					
Residential Population	1215		2741		people
Peaking factor	3.74		3.48		-
Average day residential flow	4.76		10.73		L/s
Average day industrial flow	0.00		0.00		L/s
Average day commercial/institutional flow	1.29		1.49		L/s
Average day flow; total	6.0		12.2		L/s
I/I allowance	17.2		30.5		L/s
Peak instantaneous flow; excl. I/I	22.64584		42.44910		L/s
Peak instantaneous flow; total incl. I/I	39.9	28.0	73.0	61.1	L/s

Municipality of Kincardine
SewerCAD Modelling for Master Plan
Kincardine - Calculations and Notes

Job # : 22128 Date : July 11, 2022

Revised: December 13, 2022

1.0 Background

The Municipality of Kincardine is updating its water and wastewater Master Plan. The original Master Plan was completed under BMROSS Project No. 16130. The sewage servicing component will include a review of servicing existing development and future development. The purpose of these notes is to summarize data used to create a SewerCAD model, and the results of that modelling.

1.1 References

- 1 22128 Kincardine Area SPS Catchments: Flow Notes for Master Plan
- 2 <u>08055 17Aug29-Kincardine BP MP Sec. 4.4.9.1</u>

2.0 Analysis & Model Data

2.1 Data

Re	<u>eference</u>	<u>Item</u>	
a.	1	Connaught SPS Catchment Area	
		Ex. peak sewage flow Ex. I&I allowance Ex. calculated total peak flow Ex. "observed" peak flow	24.0 L/s 14.1 L/s 38.1 L/s 68.6 L/s
b.	1	Fut. peak sewage flow Fut. I&I allowance Future 1 total peak flow Future 2 total peak flow Durham SPS Catchment Area	40.2 L/s 19.4 L/s 59.6 L/s 90.0 L/s
		Ex. peak sewage flow Ex. I&I allowance Ex. calculated total peak flow Ex. "observed" peak flow Fut. peak sewage flow Fut. I&I allowance	40.3 L/s 14.8 L/s 55.1 L/s 39.6 L/s Used 18033 DB
		Fut. total peak flow	83.0 L/s

c. 1 Huron Terrace SPS Catchment Area (w/o Connaught, Durham SPSs)

Ex. peak sewage flow Ex. I&I allowance Ex. calculated total peak flow	56.6 L/s 28.7 L/s 85.3 L/s
Fut. peak sewage flow Fut. I&I allowance Fut. calculated total peak flow	88.4 L/s 41.6 L/s 130.0 L/s

Huron Terrace SPS Catchment Area (with Connaught, Durham SPSs)

Ex. calculated total peak flow	178.6 L/s
Ex. "observed" peak flow	274.1 L/s
Future 1 total peak flow	272.5 L/s
Future 2 total peak flow	368.0 L/s

d. 1 Park SPS Catchment Area

Ex. peak sewage flow	37.1 L/s
Ex. I&I allowance	17.4 L/s
Ex. calculated total peak flow	54.5 L/s
Ex. "observed" peak flow	97 L/s
Fut. peak sewage flow	122.7 L/s
Fut. I&I allowance	36.6 L/s

Future 1 total peak flow 159.3 L/s
Future 2 total peak flow 201.3 L/s

e. 1 Goderich SPS Catchment Area

Ex. peak sewage flow Ex. I&I allowance Ex. calculated total peak flow Ex. "observed" peak flow	39.2 L/s 21.7 L/s 60.9 L/s 29.0 L/s
Fut. peak sewage flow Fut. I&I allowance Future 1 total peak flow Future 2 total peak flow	40.9 L/s 21.7 L/s 62.6 L/s 30.7 L/s

f. 1 Kincardine SPS Catchment Area

Ex. peak sewage flow	22.6 L/s
Ex. I&I allowance	17.2 L/s
Ex. calculated total peak flow	39.8 L/s
Ex. "observed" peak flow	28.0 L/s
Fut. peak sewage flow	42.5 L/s
Fut. I&I allowance	30.5 L/s
Future 1 total peak flow	73.0 L/s
Future 2 total peak flow	61 L/s
Future 2 total peak flow	61 I /s

2.2 Sewage Flows by SMH

For the existing system model, sewage flows to each sanitary maintenance hole (SMH) are calculated by dividing total peak flow for the catchment area by the number of SMHs.

For future flows, the sewage flow that is additional to existing is assigned to specific SMHs based on future service area location in relation to existing SMHs.

	Reference	<u>Item</u>			
	1 1 1 1	Flow per ERU Industrial flow allowance Commercial flow allowance Institutional flow allowance I-I allowance	0.405 L 0.324 L 0.324 L	./ERU/s ./ha/s ./ha/s ./ha/s ./ha/s	
a.	1	Connaught SPS Catchment A	Area		
		Ex. No. of SMHs in model Ex. Peak flow per SMH Ex. Peaking Factor Future Peaking Factor	111 S 0.618 L 3.708 3.489	SMHs _/s/SMH	
		Additional future peak flow	21.4 L	/s	
		from Westridge Res. ERUs IC	I 0.0 L	/s /s at	SMH-921
		Shepherd Res. ERUs IC I-	0.0 L	√s √s at	SMH-593
		Golf Links Res. ERUs IC I-	0.0 L	/s /s at	SMH-913
		Battler Res. ERUs IC I-	0.0 L	√s √s at	SMH-913
		Net: Infill and Peak Reduction Infill Res. ERUs PF Reduction of existing flow	1.9 L	/s at	All MHs
		Check:	21.8 L	/s	

b. Durham SPS Catchment Area

Ex. No. of SMHs in model 73 SMHs
Ex. Peak flow per SMH 0.755 L/s/SMH

Ex. Peaking Factor 3.800

Future Peaking Factor Not broken down. Future flows from 18033.

Additional future peak flow 27.9 L/s

Assume all applied to SMH-76A

c. Huron Terrace SPS Catchment Area (w/o Connaught, Durham SPSs)

Ex. No. of SMHs in model 133 SMHs
Ex. Peak flow per SMH 1.131 L/s/SMH

Ex. Peaking Factor 3.676 Future Peaking Factor 3.319

For Connaught, add to gravity sewer on Huron Terrace per 16171 design (SMH-495)

Existing: 68.6 L/s

For Durham, add to gravity sewer at Durham & Princess (SMH-469)

Existing: 55.1 L/s

Additional future peak flow
Assume:
Additional to SMH-495
Additional to SMH-469

Balance of Additional

93.9 L/s

21.4 L/s

27.9 L/s

44.6 L/s

from **OPF Lands** 45.8 L/s

Res. ERUs 30.9 L/s at SMH-762, ICI 3.0 L/s SMH-496

1182 Queen St. 1.1 L/s

Res. ERUs 0.4 L/s at SMH-758 ICI 0.0 L/s l-I 0.7 L/s

7 Mount Forest Ave. 1.2 L/s

Res. ERUs 0.8 L/s at SMH-759

I-I 0.4 L/s

Net: Infill and Peak Reduction
Infill Res. ERUs
-3.2 L/s
2.0 L/s

iffilii Res. EROs 2.0 L/s at All MHs

PF Reduction of existing flow -5.3 L/s

Check 44.9 L/s

d.	Park SPS Catchment Area			Page 5 of 29
	Ex. No. of SMHs in model Ex. Peak flow per SMH Ex. Peaking Factor Future Peaking Factor	146 SMHs 0.661 L/s/SMH 3.529 3.295		
	Additional future peak flow from	104.8 L/s		
	Campbell Ave	0.4 L/s		
	Res. ERUs	0.1 L/s		
	ICI	0.0 L/s	at	SMH-128
	I-I	0.2 L/s		
	Hwy. 9 & 21 Business Park	98.2 L/s	at	SMH-S16, SMH-S17, SMH-S18, SMH-565, MH-932,
	Res. ERUs	17.7 L/s		MH-S20,
	ICI	61.5 L/s		MH-S23,
	I-I	19.0 L/s		MH-S24
	7 Millennium Way Res. ERUs ICI	5.6 L/s 5.5 L/s 0.0 L/s	at	SMH-384
	I-I	0.1 L/s		
	Net: Infill and Peak Reduction Infill Res. ERUs	0.6 L/s 3.0 L/s	at	All MHs
	PF Reduction of existing flow	-2.4 L/s		
	Check:	104.8 L/s		
e.	Goderich SPS Catchment Area			
	Ex. No. of SMHs in model Ex. Peak flow per SMH Ex. Peaking Factor Future Peaking Factor	119 SMHs 0.244 L/s/SMH 3.715 3.684		
	Additional future peak flow from	33.6 L/s		
	Net: Infill, peak, method adjustm Infill Res. ERUs	33.6 L/s 2.0 L/s		
	PF Reduction of existing flow	-0.3 L/s	at	All MHs
	Calc./Obs. Adjustment	31.9 L/s		

Ex. No. of SMHs in model Ex. Peak flow per SMH Ex. Peaking Factor Future Peaking Factor	105 SMHs 0.379 L/s/SMH 3.744 3.475		
Additional future peak flow from	33.1 L/s		
Brown	10.8 L/s		
Res. ERUs	8.3 L/s		
ICI	0.0 L/s	at	SMH-832
I-I	2.5 L/s		
Brigadoon	13.5 L/s		
Res. ERUs	5.1 L/s	at	01411.000
ICI	0.0 L/s		SMH-380
I-I	8.4 L/s		
CR Developers	4.0 L/s		
Res. ERUs	2.8 L/s	at	SMH-380
ICI	0.7 L/s	aı	31VII 1-30U
I-I	0.6 L/s		
869 Kincardine Ave.	4.8 L/s		
Res. ERUs	3.0 L/s	at	SMH-380
ICI	0.0 L/s	a.	OWN 1 COC
I-I	1.8 L/s		
Net: Infill and Peak Reduction	0.2 L/s		
Infill Res. ERUs	1.7 L/s		
IIIIII Res. ERUS	1.7 4/5	at	All MHs
PF Reduction of existing flow	-1.5 L/s		
Check:	33.3 L/s		

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·	<u>.</u>					
a.	1	Connaught S	PS Catchment Area			
		· ·	Model Input Mod	del Output Diffe	erence	
		Existing	68.6	68.6	0.0%	
		Future	90.0	90.4	0.4%	
b.	1	Durham SPS	Catchment Area			
			Model Input Mod	del Output		
		Existing	55.1	55.1	0.0%	
		Future	83.0	83.0	0.0%	
C.	1	Huron Terrac	e SPS Catchment Ar	ea (w/ Connau	ght, Durham SPS	s)
			Model Input Mod	del Output		
		Existing	274.1	274.1	0.0%	
		Future	368.0	368.4	0.1%	
d.	1	Park SPS Ca	tchment Area			
			Model Input Mod	del Output		

		Future	201.3	143.8
e.	1	Goderich SPS Catchi	ment Area	

 Model Input
 Model Output

 Existing
 29.0
 29.0
 0.1%

 Future
 62.6
 62.6
 0.0%

97

96.5

0.0%

28.6%

f. 1 Kincardine SPS Catchment Area

Existing

3.0

Validation

ranoaranno or	o catominome i	ou	
	Model Input	Model Output	
Existing	39.8	39.8	0.1%
Future	73.0	72.8	0.2%

				Con	naught SPS Res	sults				
Existing Conditions Future Conditions							าร			
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
CO-1	72.919	0.001	450	68.6	94	72.9	0.001	450	90.4	124
CO-2	141.519	0.057	250	17.9	13	141.5	0.057	250	18.2	13
CO-3	92.298	0.009	300	4.3	5	92.3	0.009	300	4.4	5
CO-4	92.741	0.003	375	22.9	25	92.7	0.003	375	23.2	25
CO-5	79.125	0.002	375	23.5	30	79.1	0.002	375	23.8	30
CO-6	236.5	0.007	450	44.5	19	236.5	0.007	450	65.9	28
CO-7	122.1	0.002	450	43.9	36	122.1	0.002	450	65.3	54
CO-8	105.5	0.001	450	43.3	41	105.5	0.001	450	64.6	61
CO-9	142.6	0.003	450	42.6	30	142.6	0.003	450	64.0	45
CO-10	107.1	0.001	450	37.1	35	107.1	0.001	450	58.4	55
CO-11	100.0	0.001	450	36.5	37	100.0	0.001	450	57.7	58
CO-12	112.1	0.002	450	35.8	32	112.1	0.002	450	57.1	51
SM-9	89.9	0.002	400	33.4	37	89.9	0.002	400	54.6	61
SM-10	136.0	0.004	400	32.8	24	136.0	0.004	400	54.0	40
SM-101	28.5	0.008	200	0.6	2	28.5	0.008	200	0.6	2
SM-102	73.0	0.05	200	1.9	3	73.0	0.050	200	1.9	3
SM-104	61.5	0.004	300	1.2	2	61.5	0.004	300	1.3	2
SM-108	34.1	0.001	300	1.9	5	34.1	0.001	300	1.9	6
SM-109	43.1	0.002	300	2.5	6	43.1	0.002	300	2.5	6
SM-110	42.2	0.002	300	3.1	7	42.2	0.002	300	3.1	7
SM-111	46.7	0.002	300	3.7	8	46.7	0.002	300	3.8	8
SM-113	13.4	0.002	200	0.6	5	13.4	0.002	200	0.6	5
SM-116	20.2	0.004	200	1.2	6	20.2	0.004	200	1.3	6
SM-117	19.9	0.004	200	1.9	9	19.9	0.004	200	1.9	10
SM-119	33.1	0.003	250	15.5	47	33.1	0.003	250	15.7	47
SM-120	27.3	0.002	250	16.1	59	27.3	0.002	250	16.3	60
SM-121	22.4	0.005	200	4.3	19	22.4	0.005	200	4.4	20
SM-122	21.3	0.004	200	4.9	23	21.3	0.004	200	5.0	24
SM-123	19.2	0.003	200	5.6	29	19.2	0.003	200	5.6	29
SM-124	20.3	0.004	200	6.2	31	20.3	0.004	200	6.3	31
SM-125	26.4	0.006	200	1.2	5	26.4	0.006	200	1.3	5
SM-127	35.0	0.003	250	4.3	12	35.0	0.003	250	4.4	13
SM-128	23.3	0.005	200	2.5	11	23.3	0.005	200	2.5	11
SM-129	80.9	0.061	200	0.6	1	80.9	0.061	200	0.6	1
SM-130	108.5	0.109	200	2.5	2	108.5	0.109	200	2.5	2
SM-135	28.6	0.008	200	0.6	2	28.6	0.008	200	0.6	2

Connaught SPS Results										
		E	xisting Conditio	ns	Future Conditions					
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-137	32.2	0.01	200	0.6	2	32.2	0.010	200	0.6	2
SM-138	40.2	0.015	200	0.6	2	40.2	0.015	200	0.6	2
SM-139	30.2	0.008	200	2.5	8	30.2	0.008	200	2.5	8
SM-140	106.3	0.105	200	3.7	4	106.3	0.105	200	3.8	4
SM-141	35.3	0.004	250	9.3	26	35.3	0.004	250	9.4	27
SM-142	39.1	0.004	250	14.8	38	39.1	0.004	250	15.0	39
SM-143	34.8	0.003	250	4.9	14	34.8	0.003	250	5.0	14
SM-144	118.2	0.13	200	0.6	1	118.2	0.130	200	0.6	1
SM-145	47.146	0.021	200	1.2	3	47.1	0.021	200	1.3	3
SM-146	74.3	0.051	200	0.6	1	74.3	0.051	200	0.6	1
SM-148	48.3	0.022	200	1.2	3	48.3	0.022	200	1.3	3
SM-150	24.0	0.005	200	1.9	8	24.0	0.005	200	1.9	8
SM-152	35.3	0.012	200	4.9	14	35.3	0.012	200	5.0	14
SM-153	52.6	0.003	300	0.6	1	52.6	0.003	300	0.6	1
SM-154	79.5	0.059	200	4.3	5	79.5	0.059	200	4.4	6
SM-156	79.7	0.059	200	3.7	5	79.7	0.059	200	3.8	5
SM-157	39.9	0.015	200	3.1	8	39.9	0.015	200	3.1	8
SM-158	40.1	0.015	200	0.6	2	40.1	0.015	200	0.6	2
SM-160	27.8	0.007	200	0.6	2	27.8	0.007	200	0.6	2
SM-161	98.2	0.09	200	1.2	1	98.2	0.090	200	1.3	1
SM-162	33.4	0.003	250	16.7	50	33.4	0.003	250	16.9	51
SM-163	29.2	0.002	250	17.3	59	29.2	0.002	250	17.6	60
SM-238	36.2	0.012	200	2.5	7	36.2	0.012	200	4.3	12
SM-239	21.7	0.004	200	3.1	14	21.7	0.004	200	4.9	23
SM-287	48.3	0.022	200	0.6	1	48.3	0.022	200	0.6	1
SM-288	26.7	0.007	200	1.2	5	26.7	0.007	200	1.3	5
SM-289	30.1	0.008	200	1.9	6	30.1	0.008	200	1.9	6
SM-432	30.0	0.008	200	0.6	2	30.0	0.008	200	0.6	2
SM-450	103.2	0.003	375	7.4	7	103.2	0.003	375	24.9	24
SM-567	160.1	0.008	375	18.5	12	160.1	0.008	375	37.8	24
SM-572	18.7	0.003	200	1.9	10	18.7	0.003	200	1.9	10
SM-615	30.9	0.009	200	1.2	4	30.9	0.009	200	1.3	4
SM-616	29.1	0.008	200	1.9	6	29.1	0.008	200	1.9	7
SM-617	22.5	0.005	200	0.6	3	22.5	0.005	200	0.6	3
SM-618	19.2	0.003	200	3.1	16	19.2	0.003	200	3.1	16
SM-620	28.2	0.007	200	0.6	2	28.2	0.007	200	0.6	2

Connaught SPS Results										
	Existing Conditions						Future Conditions			
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-621	26.5	0.007	200	3.7	14	26.5	0.007	200	3.8	14
SM-622	40.1	0.015	200	0.6	2	40.1	0.015	200	0.6	2
SM-623	25.9	0.006	200	0.6	2	25.9	0.006	200	0.6	2
SM-624	35.8	0.012	200	1.9	5	35.8	0.012	200	1.9	5
SM-625	34.8	0.011	200	2.5	7	34.8	0.011	200	2.5	7
SM-627	39.1	0.014	200	1.9	5	39.1	0.014	200	3.5	9
SM-628	43.3	0.017	200	2.5	6	43.3	0.017	200	4.1	10
SM-629	27.2	0.007	200	3.1	11	27.2	0.007	200	4.8	18
SM-630	21.8	0.004	200	7.4	34	21.8	0.004	200	9.1	42
SM-631	25.0	0.006	200	0.6	3	25.0	0.006	200	2.3	9
SM-632	23.9	0.005	200	1.2	5	23.9	0.005	200	2.9	12
SM-633	42.5	0.017	200	0.6	2	42.5	0.017	200	0.6	2
SM-634	25.2	0.006	200	1.2	5	25.2	0.006	200	1.3	5
SM-635	22.0	0.004	200	1.9	8	22.0	0.004	200	1.9	9
SM-636	24.3	0.005	200	2.5	10	24.3	0.005	200	2.5	10
SM-637	31.3	0.009	200	10.5	34	31.3	0.009	200	12.3	39
SM-734	110.0	0.004	375	30.3	28	110.0	0.004	375	51.5	47
SM-735	94.7	0.003	375	30.9	33	94.7	0.003	375	52.1	55
SM-736	108.0	0.004	375	31.5	29	108.0	0.004	375	52.7	49
SM-737	96.4	0.003	375	32.1	33	96.4	0.003	375	53.4	55
SM-778	35.0	0.011	200	1.9	5	35.0	0.011	200	3.6	10
SM-779	173.6	0.01	375	29.7	17	173.6	0.010	375	50.9	29
SM-780	177.5	0.01	375	29.0	16	177.5	0.010	375	50.2	28
SM-781	331.6	0.036	375	28.4	9	331.6	0.036	375	49.6	15
SM-782	87.7	0.003	375	24.1	28	87.7	0.003	375	45.2	52
SM-783	92.6	0.003	375	23.5	25	92.6	0.003	375	44.6	48
SM-784	79.7	0.059	200	3.7	5	79.7	0.059	200	5.5	7
SM-789	121.6	0.005	375	6.8	6	121.6	0.005	375	24.3	20
SM-790	105.9	0.004	375	19.2	18	105.9	0.004	375	38.4	36
SM-843	37.5	0.013	200	3.7	10	37.5	0.013	200	3.8	10
SM-929	29.3	0.002	250	0.0	0	29.3	0.002	250	0.0	0
SM-931	51.4	0.025	200	1.2	2	51.4	0.025	200	3.0	6
SM-932	53.8	0.027	200	0.6	1	53.8	0.027	200	2.4	4
SM-933	19.1	0.003	200	2.5	13	19.1	0.003	200	2.5	13
SM-935	29.7	0.002	250	0.6	2	29.7	0.002	250	0.6	2
SM-936	104.6	0.004	375	5.6	5	104.6	0.004	375	23.0	22

	Connaught SPS Results										
Existing Conditions					Future Conditions						
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	
SM-937	119.3	0.005	375	1.2	1	119.3	0.005	375	18.6	16	
SM-938	104.5	0.004	375	0.6	1	104.5	0.004	375	18.0	17	
SM-939	98.8	0.091	200	3.7	4	98.8	0.091	200	3.8	4	
SM-948	23.2	0.005	200	0.6	3	23.2	0.005	200	0.6	3	

Denotes greater than 80% capacity utilized.

Denotes greater than 100% capacity utilized (i.e. surcharging).

Durham SPS Results

1		E	xisting Conditio	ns			F	uture Condition	าร	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
CO-1	25.3	0.006	200	0.0	0	25.3	0.006	200	0.0	0
CO-2	67.2	0.005	300	11.3	17	67.2	0.005	300	39.2	58.4
CO-3	85.0	0.008	300	10.6	12	85.0	0.008	300	38.5	45.2
CO-4	79.7	0.007	300	9.8	12	79.7	0.007	300	37.7	47.3
CO-5	69.2	0.005	300	6.0	9	69.2	0.005	300	33.9	49.0
CO-6	63.3	0.004	300	5.3	8	63.3	0.004	300	33.2	52.4
CO-7	95.0	0.010	300	4.5	5	95.0	0.010	300	32.4	34.1
CO-8	72.4	0.006	300	3.8	5	72.4	0.006	300	31.7	43.7
CO-9	75.5	0.006	300	1.5	2	75.5	0.006	300	29.4	38.9
CO-10	78.5	0.007	300	0.8	1	78.5	0.007	300	28.6	36.5
CO-11	82.6	0.007	300	19.6	24	82.6	0.007	300	47.5	57.5
SM-7	83.6	0.065	200	0.8	1	83.6	0.065	200	0.8	0.9
SM-11	20.7	0.004	200	5.3	26	20.7	0.004	200	5.3	25.5
SM-12	21.5	0.004	200	6.0	28	21.5	0.004	200	6.0	28.1
SM-14	23.0	0.005	200	6.8	30	23.0	0.005	200	6.8	29.6
SM-15	58.4	0.032	200	7.6	13	58.4	0.032	200	7.6	12.9
SM-166	41.0	0.016	200	0.8	2	41.0	0.016	200	0.8	1.8
SM-167	28.9	0.008	200	1.5	5	28.9	0.008	200	1.5	5.2
SM-168	25.3	0.006	200	0.0	0	25.3	0.006	200	0.0	0.0
SM-169	36.0	0.012	200	0.8	2	36.0	0.012	200	0.8	2.1
SM-172	23.8	0.005	200	1.5	6	23.8	0.005	200	1.5	6.3
SM-178	20.1	0.004	200	3.0	15	20.1	0.004	200	3.0	15.0
SM-179	21.3	0.004	200	0.0	0	21.3	0.004	200	0.0	0.0
SM-182	30.8	0.009	200	0.8	2	30.8	0.009	200	0.8	2.4
SM-183	30.5	0.009	200	1.5	5	30.5	0.009	200	1.5	4.9
SM-184	31.7	0.009	200	2.3	7	31.7	0.009	200	2.3	7.1
SM-185	37.0	0.013	200	3.0	8	37.0	0.013	200	3.0	8.2
SM-186	36.7	0.012	200	3.8	10	36.7	0.012	200	3.8	10.3
SM-187	33.2	0.010	200	4.5	14	33.2	0.010	200	4.5	13.7
SM-188	20.5	0.004	200	0.8	4	20.5	0.004	200	0.8	3.7
SM-189	23.1	0.005	200	1.5	7	23.1	0.005	200	1.5	6.5
SM-190	20.7	0.004	200	2.3	11	20.7	0.004	200	2.3	10.9
SM-191	20.4	0.004	200	3.0	15	20.4	0.004	200	3.0	14.8
SM-194	22.4	0.005	200	3.8	17	22.4	0.005	200	3.8	16.8
SM-195	148.2	0.007	375	20.4	14	148.2	0.007	375	48.3	32.6
SM-196	126.2	0.005	375	21.1	17	126.2	0.005	375	49.0	38.8
SM-197	228.8	0.017	375	21.9	10	228.8	0.017	375	49.8	21.8

Durham SPS Results

		E	xisting Conditio	ns			F	uture Condition	าร	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-199	104.5	0.004	375	26.4	25	104.5	0.004	375	54.3	52.0
SM-200	119.8	0.005	375	27.2	23	119.8	0.005	375	55.1	46.0
SM-201	26.8	0.007	200	0.8	3	26.8	0.007	200	0.8	2.8
SM-202	31.7	0.009	200	2.3	7	31.7	0.009	200	2.3	7.1
SM-203	22.9	0.005	200	4.5	20	22.9	0.005	200	4.5	19.8
SM-204	22.2	0.005	200	5.3	24	22.2	0.005	200	5.3	23.8
SM-205	21.4	0.004	200	6.0	28	21.4	0.004	200	6.0	28.2
SM-206	51.2	0.024	200	3.0	6	51.2	0.024	200	3.0	5.9
SM-207	124.0	0.005	375	35.5	29	124.0	0.005	375	63.4	51.1
SM-208	190.1	0.012	375	27.9	15	190.1	0.012	375	55.8	29.4
SM-209	194.2	0.012	375	28.7	15	194.2	0.012	375	56.6	29.1
SM-210	45.6	0.019	200	1.5	3	45.6	0.019	200	1.5	3.3
SM-211	32.9	0.010	200	2.3	7	32.9	0.010	200	2.3	6.9
SM-212	124.0	0.005	375	39.3	32	124.0	0.005	375	67.1	54.1
SM-213	123.6	0.005	375	40.0	32	123.6	0.005	375	67.9	54.9
SM-214	22.8	0.005	200	0.8	3	22.8	0.005	200	0.8	3.3
SM-216	25.8	0.006	200	0.8	3	25.8	0.006	200	0.8	2.9
SM-217	52.5	0.026	200	0.0	0	52.5	0.026	200	0.0	0.0
SM-218	30.5	0.009	200	0.8	3	30.5	0.009	200	0.8	2.5
SM-219	20.2	0.004	200	0.8	4	20.2	0.004	200	0.8	3.7
SM-220	22.0	0.005	200	1.5	7	22.0	0.005	200	1.5	6.9
SM-221	19.8	0.004	200	1.5	8	19.8	0.004	200	1.5	7.6
SM-222	56.4	0.030	200	0.8	1	56.4	0.030	200	0.8	1.3
SM-223	24.8	0.006	200	3.8	15	24.8	0.006	200	3.8	15.2
SM-224	15.3	0.002	200	4.5	30	15.3	0.002	200	4.5	29.7
SM-225	20.2	0.004	200	5.3	26	20.2	0.004	200	5.3	26.2
SM-226	21.5	0.004	200	7.6	35	21.5	0.004	200	7.6	35.1
SM-228	25.2	0.006	200	2.3	9	25.2	0.006	200	2.3	9.0
SM-229	25.2	0.006	200	0.8	3	25.2	0.006	200	0.8	3.0
SM-230	122.1	0.005	375	41.5	34	122.1	0.005	375	69.4	56.8
SM-231	135.6	0.006	375	42.3	31	135.6	0.006	375	70.2	51.7
SM-232	160.8	0.008	375	43.0	27	160.8	0.008	375	70.9	44.1
SM-233	60.5	0.034	200	0.8	1	60.5	0.034	200	0.8	1.2
SM-234	44.3	0.018	200	9.8	22	44.3	0.018	200	9.8	22.1
SM-235	72.1	0.048	200	10.6	15	72.1	0.048	200	10.6	14.7
SM-236	356.9	0.041	375	55.1	15	356.9	0.041	375	83.0	23.3

Durham SPS Results

		E	xisting Conditio	าร			F	uture Condition	S	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-237	20.8	0.004	200	8.3	40	20.8	0.004	200	8.3	39.9
SM-275	28.7	0.008	200	3.8	13	28.7	0.008	200	3.8	13.2
SM-777	40.4	0.015	200	0.8	2	40.4	0.015	200	0.8	1.9
SM-802	252.9	0.021	375	43.8	17	252.9	0.021	375	71.7	28.3

Notes:

Denotes greater than 80% capacity utilized.

Denotes greater than 100% capacity utilized (i.e. surcharging).

Huron Terrace SPS Results

		E	xisting Conditio	ns			F	uture Condition	ns	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
CO-1	401.839	0.004	600	203.4	50.6	401.8	0.004	600	271.2	67.5
CO-2	387.6	0.004	600	204.5	52.8	387.6	0.004	600	272.3	70.2
CO-3	551.1	0.008	600	218.7	39.7	551.1	0.008	600	314.1	57.0
CO-4	1130.4	0.034	600	219.8	19.4	1130.4	0.034	600	315.2	27.9
CO-5	513.0	0.007	600	11.3	2.2	513.0	0.007	600	11.1	2.2
CO-6	589.4	0.009	600	12.4	2.1	589.4	0.009	600	12.2	2.1
CO-10	489.8	0.006	600	1.1	0.2	489.8	0.006	600	1.1	0.2
CO-11	916.6	0.022	600	0.0	0.0	916.6	0.022	600	0.0	0.0
CO-12	447.1	0.005	600	2.3	0.5	447.1	0.005	600	2.2	0.5
CO-13				Introduced a	as a suggested u	pgrade in 2018	Master Plan.			
SM-97	341.5	0.027	400	274.1	80.3	341.5	0.027	400	368.4	107.9
SM-241	25.7	0.006	200	6.8	26.4	25.7	0.006	200	31.8	123.9
SM-242	26.0	0.006	200	9.0	34.8	26.0	0.006	200	34.1	131.1
SM-243	18.8	0.003	200	10.2	54.1	18.8	0.003	200	35.2	187.0
SM-244	21.0	0.004	200	30.5	145.2	21.0	0.004	200	78.0	370.7
SM-245	26.7	0.007	200	31.7	118.5	26.7	0.007	200	79.1	296.0
SM-246	18.5	0.003	200	32.8	177.4	18.5	0.003	200	80.2	433.9
SM-247	18.2	0.003	200	50.9	279.3	18.2	0.003	200	97.9	537.3
SM-248	26.6	0.007	200	52.0	195.6	26.6	0.007	200	99.0	372.3
SM-249	23.0	0.005	200	1.1	4.9	23.0	0.005	200	1.1	4.8
SM-250	21.6	0.004	200	0.0	0.0	21.6	0.004	200	0.0	0.0
SM-251	20.9	0.004	200	1.1	5.4	20.9	0.004	200	1.1	5.3
SM-256	173.9	0.010	375	5.7	3.3	173.9	0.010	375	5.5	3.2
SM-257	35.1	0.003	250	10.2	29.0	35.1	0.003	250	10.0	28.4
SM-258	37.3	0.004	250	9.0	24.3	37.3	0.004	250	8.9	23.8
SM-259	18.7	0.003	200	0.0	0.0	18.7	0.003	200	0.0	0.0
SM-260	20.8	0.004	200	1.1	5.4	20.8	0.004	200	1.1	5.3
SM-261	21.5	0.004	200	2.3	10.5	21.5	0.004	200	2.2	10.3
SM-262	19.8	0.004	200	1.1	5.7	19.8	0.004	200	1.1	5.6
SM-263	41.5	0.005	250	4.5	10.9	41.5	0.005	250	4.4	10.7
SM-264	39.0	0.004	250	6.8	17.4	39.0	0.004	250	6.6	17.0
SM-265	20.5	0.004	200	3.4	16.6	20.5	0.004	200	3.3	16.2
SM-266	20.5	0.004	200	2.3	11.0	20.5	0.004	200	2.2	10.8
SM-267	23.8	0.005	200	1.1	4.8	23.8	0.005	200	1.1	4.7
SM-270	27.1	0.007	200	2.3	8.4	27.1	0.007	200	2.2	8.2
SM-326	19.8	0.004	200	1.1	5.7	19.8	0.004	200	1.1	5.6
SM-332	38.9	0.014	200	1.1	2.9	38.9	0.014	200	1.1	2.8
SM-333	21.9	0.004	200	4.5	20.7	21.9	0.004	200	27.3	124.9
SM-334	48.9	0.022	200	1.1	2.3	48.9	0.022	200	1.1	2.3

Huron Terrace SPS Results

		E	xisting Conditio	ns			F	uture Condition	ıs	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-335	22.5	0.005	200	2.3	10.1	22.5	0.005	200	2.2	9.9
SM-336	34.0	0.011	200	4.5	13.3	34.0	0.011	200	4.4	13.0
SM-337	19.8	0.004	200	10.2	51.3	19.8	0.004	200	32.9	165.6
SM-338	18.7	0.003	200	11.3	60.3	18.7	0.003	200	34.0	181.2
SM-339	22.8	0.005	200	1.1	5.0	22.8	0.005	200	1.1	4.9
SM-340	53.3	0.026	200	2.3	4.2	53.3	0.026	200	2.2	4.2
SM-341	33.2	0.010	200	14.7	44.3	33.2	0.010	200	37.3	112.4
SM-342	62.3	0.036	200	1.1	1.8	62.3	0.036	200	1.1	1.8
SM-343	40.5	0.015	200	2.3	5.6	40.5	0.015	200	2.2	5.5
SM-344	32.8	0.010	200	3.4	10.3	32.8	0.010	200	3.3	10.1
SM-345	28.6	0.008	200	4.5	15.8	28.6	0.008	200	4.4	15.5
SM-346	26.3	0.006	200	1.1	4.3	26.3	0.006	200	1.1	4.2
SM-347	18.5	0.003	200	6.8	36.7	18.5	0.003	200	6.6	35.9
SM-348	14.4	0.002	200	7.9	55.0	14.4	0.002	200	7.7	53.8
SM-349	21.4	0.004	200	5.7	26.5	21.4	0.004	200	5.5	25.9
SM-350	49.4	0.023	200	4.5	9.1	49.4	0.023	200	4.4	9.0
SM-353	21.2	0.004	200	18.1	85.3	21.2	0.004	200	40.6	191.5
SM-354	63.1	0.037	200	1.1	1.8	63.1	0.037	200	1.1	1.8
SM-355	22.4	0.005	200	15.8	70.8	22.4	0.005	200	15.5	69.3
SM-356	51.7	0.025	200	0.0	0.0	51.7	0.025	200	0.0	0.0
SM-357	46.2	0.020	200	1.1	2.4	46.2	0.020	200	1.1	2.4
SM-358	33.5	0.010	200	2.3	6.7	33.5	0.010	200	2.2	6.6
SM-359	41.0	0.016	200	1.1	2.8	41.0	0.016	200	1.1	2.7
SM-360	38.2	0.014	200	2.3	5.9	38.2	0.014	200	2.2	5.8
SM-361	33.7	0.011	200	56.6	167.9	33.7	0.011	200	103.4	307.1
SM-362	237.1	0.003	525	12.4	5.2	237.1	0.003	525	12.2	5.1
SM-363	109.2	0.001	525	13.6	12.4	109.2	0.001	525	13.3	12.2
SM-364	274.9	0.004	525	17.0	6.2	274.9	0.004	525	16.6	6.0
SM-365	142.5	0.001	525	18.1	12.7	142.5	0.001	525	17.7	12.4
SM-366	280.6	0.004	525	144.4	51.4	280.6	0.004	525	212.3	75.7
SM-367	282.3	0.004	525	145.5	51.5	282.3	0.004	525	213.4	75.6
SM-368	285.6	0.004	525	146.6	51.3	285.6	0.004	525	214.5	75.1
SM-369	85.8	0.021	250	54.5	63.5	85.8	0.021	250	54.4	63.4
SM-370	84.0	0.020	250	51.1	60.9	84.0	0.020	250	51.1	60.8
SM-371	111.3	0.035	250	50.0	44.9	111.3	0.035	250	50.0	44.9
SM-372	22.9	0.005	200	6.3	27.3	22.9	0.005	200	34.1	148.8
SM-373	12.3	0.001	200	7.4	60.0	12.3	0.001	200	35.2	286.0
SM-374	61.9	0.036	200	8.5	13.8	61.9	0.036	200	36.3	58.7
SM-375	51.1	0.024	200	9.6	18.9	51.1	0.024	200	37.4	73.2

		E	xisting Conditio	ns			F	uture Condition		
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-376	51.2	0.024	200	10.8	21.1	51.2	0.024	200	38.5	75.3
SM-377	42.7	0.017	200	11.9	27.9	42.7	0.017	200	39.6	92.9
SM-378	32.2	0.010	200	13.0	40.5	32.2	0.010	200	40.7	126.6
SM-379	42.1	0.005	250	55.7	132.1	42.1	0.005	250	55.5	131.9
SM-382	19.2	0.003	200	1.1	5.9	19.2	0.003	200	1.1	5.8
SM-383	34.1	0.011	200	2.3	6.6	34.1	0.011	200	2.2	6.5
SM-384	65.8	0.040	200	3.4	5.2	65.8	0.040	200	3.3	5.0
SM-385	19.5	0.004	200	0.0	0.0	19.5	0.004	200	0.0	0.0
SM-386	24.1	0.005	200	1.1	4.7	24.1	0.005	200	1.1	4.6
SM-387	23.6	0.005	200	1.1	4.8	23.6	0.005	200	24.0	101.7
SM-388	55.3	0.028	200	2.3	4.1	55.3	0.028	200	25.1	45.4
SM-389	41.1	0.016	200	19.2	46.8	41.1	0.016	200	41.7	101.5
SM-390	24.0	0.005	200	4.5	18.8	24.0	0.005	200	4.4	18.4
SM-391	19.5	0.004	200	6.8	34.8	19.5	0.004	200	6.6	34.1
SM-392	225.5	0.003	525	9.0	4.0	225.5	0.003	525	8.9	3.9
SM-393	38.8	0.014	200	1.1	2.9	38.8	0.014	200	1.1	2.9
SM-394	34.3	0.011	200	55.4	161.8	34.3	0.011	200	102.3	298.8
SM-395	45.7	0.019	200	1.1	2.5	45.7	0.019	200	1.1	2.4
SM-396	43.7	0.005	250	13.6	31.1	43.7	0.005	250	13.3	30.4
SM-397	103.5	0.030	250	14.7	14.2	103.5	0.030	250	14.4	13.9
SM-398	36.7	0.004	250	17.0	46.3	36.7	0.004	250	16.6	45.3
SM-399	38.6	0.004	250	18.1	46.9	38.6	0.004	250	17.7	45.9
SM-400	32.4	0.003	250	19.2	59.3	32.4	0.003	250	18.8	58.0
SM-401	38.9	0.014	200	4.5	11.6	38.9	0.014	200	4.4	11.4
SM-402	48.7	0.022	200	5.7	11.6	48.7	0.022	200	5.5	11.4
SM-403	38.9	0.004	250	26.0	66.9	38.9	0.004	250	25.5	65.5
SM-404	38.7	0.004	250	27.1	70.2	38.7	0.004	250	26.6	68.7
SM-406	92.6	0.024	250	28.3	30.5	92.6	0.024	250	27.7	29.9
SM-407	123.0	0.043	250	29.4	23.9	123.0	0.043	250	28.8	23.4
SM-408	61.8	0.011	250	30.5	49.4	61.8	0.011	250	29.9	48.4
SM-409	72.4	0.015	250	31.7	43.7	72.4	0.015	250	31.0	42.8
SM-410	1056.9	0.030	600	239.1	22.6	1056.9	0.030	600	334.0	31.6
SM-411	345.3	0.003	600	237.9	68.9	345.3	0.003	600	332.9	96.4
SM-413	139.3	0.055	250	17.0	12.2	139.3	0.055	250	16.6	11.9
SM-416	77.0	0.055	200	1.1	1.5	77.0	0.055	200	1.1	1.4
SM-417	16.2	0.002	200	5.7	35.0	16.2	0.002	200	5.5	34.3
SM-418	20.6	0.004	200	1.1	5.5	20.6	0.004	200	1.1	5.4
SM-419	29.2	0.008	200	1.1	3.9	29.2	0.008	200	1.1	3.8
SM-420	32.7	0.010	200	3.4	10.4	32.7	0.010	200	3.3	10.2

Huron Terrace SPS Results

		E	xisting Conditio	ns			F	uture Condition	ıs	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-421	44.2	0.018	200	1.1	2.6	44.2	0.018	200	1.1	2.5
SM-422	21.8	0.004	200	1.1	5.2	21.8	0.004	200	1.1	5.1
SM-423	24.0	0.005	200	2.3	9.4	24.0	0.005	200	2.2	9.2
SM-424	39.6	0.015	200	1.1	2.9	39.6	0.015	200	1.1	2.8
SM-425	42.6	0.017	200	3.4	8.0	42.6	0.017	200	3.3	7.8
SM-426	59.1	0.033	200	4.5	7.6	59.1	0.033	200	4.4	7.5
SM-427	21.2	0.004	200	0.0	0.0	21.2	0.004	200	0.0	0.0
SM-428	26.7	0.007	200	2.3	8.5	26.7	0.007	200	2.2	8.3
SM-429	24.6	0.006	200	17.0	69.0	24.6	0.006	200	16.6	67.6
SM-433	59.8	0.033	200	1.1	1.9	59.8	0.033	200	1.1	1.9
SM-434	46.5	0.020	200	2.3	4.9	46.5	0.020	200	2.2	4.8
SM-767	155.5	0.003	450	1.1	0.7	155.5	0.003	450	24.0	15.4
SM-768	156.9	0.003	450	2.3	1.4	156.9	0.003	450	25.1	16.0
SM-769	156.3	0.003	450	3.4	2.2	156.3	0.003	450	26.2	16.8
SM-770	20.4	0.004	200	4.5	22.1	20.4	0.004	200	28.5	139.6
SM-771	15.5	0.002	200	5.7	36.4	15.5	0.002	200	30.7	197.8
SM-808	49.8	0.007	250	15.8	31.8	49.8	0.007	250	15.5	31.1
SM-922	37.4	0.004	250	7.9	21.2	37.4	0.004	250	7.7	20.7
SM-944	1925.9	0.098	600	240.2	12.5	1925.9	0.098	600	335.1	17.4
SM-945	2378.4	0.150	600	241.3	10.1	2378.4	0.150	600	336.3	14.1
SM-951	39.1	0.004	250	5.7	14.5	39.1	0.004	250	5.5	14.2
SM-977	21.1	0.004	200	3.4	16.0	21.1	0.004	200	3.3	15.7
SM-978	38.6	0.014	200	2.3	5.9	38.6	0.014	200	2.2	5.7
SM-979	40.4	0.015	200	1.1	2.8	40.4	0.015	200	1.1	2.7

Notes:

Denotes greater than 80% capacity utilized.

Denotes greater than 100% capacity utilized (i.e. surcharging).

Park St. SPS Results

		Е	xisting Conditio	ns		Future Co	onditions - Rus	sell/Millennium S	Sewer Capacity	Base Case
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
CO-1	165.7	0.009	375	13.9	8.4	165.7	0.009	375	60.2	36.3
CO-2	139.0	0.006	375	13.2	9.5	139.0	0.006	375	59.6	42.8
CO-3	199.2	0.013	375	12.6	6.3	199.2	0.013	375	58.9	29.6
CO-4	154.0	800.0	375	11.9	7.7	154.0	0.008	375	58.2	37.8
CO-5	119.6	0.005	375	11.2	9.4	119.6	0.005	375	57.6	48.1
CO-6	99.7	0.003	375	10.6	10.6	99.7	0.003	375	56.9	57.0
CO-7	121.3	0.005	375	9.9	8.2	121.3	0.005	375	56.2	46.4
CO-8	86.2	0.002	375	9.3	10.7	86.2	0.002	375	55.6	64.4
CO-9	152.5	800.0	375	8.6	5.6	152.5	0.008	375	0.0	0.0
CO-10	72.6	0.006	300	7.9	10.9	72.6	0.006	300	0.0	0.0
CO-11	69.5	0.005	300	7.3	10.5	69.5	0.005	300	0.0	0.0
CO-12	69.4	0.005	300	6.6	9.5	69.4	0.005	300	0.0	0.0
CO-13	56.3	0.003	300	5.9	10.6	56.3	0.003	300	0.0	0.0
CO-14	69.2	0.005	300	5.3	7.6	69.2	0.005	300	0.0	0.0
CO-15	75.0	0.006	300	4.6	6.2	75.0	0.006	300	0.0	0.0
CO-16	100.8	0.011	300	4.0	3.9	100.8	0.011	300	0.0	0.0
CO-17	84.6	0.012	250	0.7	0.8	84.6	0.012	250	0.0	0.0
CO-18	121.6	0.025	250	1.3	1.1	121.6	0.025	250	0.0	0.0
CO-19	108.9	0.02	250	2.0	1.8	108.9	0.02	250	0.0	0.0
CO-20	73.8	0.009	250	2.6	3.6	73.8	0.009	250	0.0	0.0
CO-21	262.8	0.022	375	14.5	5.5	262.8	0.022	375	60.9	23.2
SM-2	76.2	0.054	200	0.7	0.9	76.2	0.054	200	0.7	0.9
SM-3	56.1	0.029	200	1.3	2.4	56.1	0.029	200	1.3	2.4
SM-6	75.7	0.053	200	2.0	2.6	75.7	0.053	200	2.0	2.6
SM-91	35.4	0.012	200	0.7	1.9	35.4	0.012	200	0.0	0.0
SM-95	36.7	0.004	250	0.7	1.8	36.7	0.004	250	0.7	1.8
SM-444	39.0	0.014	200	1.3	3.4	39.0	0.014	200	1.3	3.4
SM-445	40.6	0.015	200	2.0	4.9	40.6	0.015	200	2.0	4.9
SM-446	40.2	0.015	200	2.6	6.6	40.2	0.015	200	2.7	6.6
SM-447	39.4	0.014	200	3.3	8.4	39.4	0.014	200	3.3	8.4
SM-448	25.8	0.006	200	2.0	7.7	25.8	0.006	200	2.0	7.7
SM-449	25.1	0.006	200	1.3	5.3	25.1	0.006	200	1.3	5.3
SM-451	37.0	0.013	200	0.7	1.8	37.0	0.013	200	0.7	1.8
SM-452	26.5	0.007	200	0.0	0.0	26.5	0.007	200	0.0	0.0
SM-453	49.7	0.023	200	0.7	1.3	49.7	0.023	200	0.7	1.3
SM-454	25.9	0.006	200	1.3	5.1	25.9	0.006	200	1.3	5.1
SM-455	26.1	0.006	200	2.6	10.1	26.1	0.006	200	2.7	10.2
SM-456	27.2	0.007	200	3.3	12.2	27.2	0.007	200	3.3	12.2
SM-457	26.4	0.006	200	4.0	15.0	26.4	0.006	200	4.0	15.1

Park St. SPS Results

		E	xisting Conditio	ns		Future Co	onditions - Russ	sell/Millennium \$	Sewer Capacity	Base Case
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-458	26.7	0.007	200	4.6	17.3	26.7	0.007	200	4.7	17.5
SM-459	26.9	0.007	200	5.3	19.6	26.9	0.007	200	5.3	19.8
SM-460	25.5	0.006	200	5.9	23.4	25.5	0.006	200	6.0	23.5
SM-461	25.9	0.006	200	6.6	25.5	25.9	0.006	200	6.7	25.7
SM-462	26.7	0.007	200	1.3	4.9	26.7	0.007	200	1.3	5.0
SM-463	40.5	0.015	200	0.7	1.6	40.5	0.015	200	0.7	1.6
SM-464	33.3	0.01	200	5.9	17.9	33.3	0.01	200	6.0	18.0
SM-466	163.3	0.006	400	58.2	35.6	163.3	0.006	400	105.2	64.4
SM-467	144.3	0.005	400	63.5	44.0	144.3	0.005	400	110.5	76.6
SM-468	148.9	0.005	400	64.1	43.1	148.9	0.005	400	111.2	74.7
SM-469	130.6	0.004	400	80.6	61.7	130.6	0.004	400	127.8	97.9
SM-470	133.0	0.004	400	81.3	61.2	133.0	0.004	400	128.5	96.7
SM-471	141.2	0.005	400	82.0	58.0	141.2	0.005	400	129.2	91.5
SM-472	161.2	0.006	400	82.6	51.3	161.2	0.006	400	129.8	80.5
SM-473	137.4	0.004	400	85.3	62.1	137.4	0.004	400	132.5	96.5
SM-474	131.2	0.004	400	85.9	65.5	131.2	0.004	400	133.2	101.5
SM-475	121.9	0.003	400	86.6	71.0	121.9	0.003	400	133.8	109.8
SM-476	115.2	0.003	400	87.3	75.7	115.2	0.003	400	134.5	116.7
SM-477	175.4	0.004	450	95.8	54.7	175.4	0.004	450	143.2	81.6
SM-478	171.1	0.004	450	96.5	56.4	171.1	0.004	450	143.8	84.1
SM-479	26.4	0.006	200	0.7	2.5	26.4	0.006	200	0.7	2.5
SM-480	16.7	0.003	200	14.5	87.0	16.7	0.003	200	14.7	87.6
SM-481	27.2	0.007	200	15.9	58.3	27.2	0.007	200	16.0	58.7
SM-482	24.6	0.006	200	15.2	61.8	24.6	0.006	200	15.3	62.3
SM-483	27.1	0.007	200	2.0	7.3	27.1	0.007	200	2.0	7.4
SM-484	45.6	0.019	200	1.3	2.9	45.6	0.019	200	1.3	2.9
SM-485	22.8	0.005	200	0.7	2.9	22.8	0.005	200	0.7	2.9
SM-486	24.0	0.005	200	0.0	0.0	24.0	0.005	200	0.0	0.0
SM-487	25.7	0.006	200	0.7	2.6	25.7	0.006	200	0.7	2.6
SM-488	21.7	0.004	200	1.3	6.1	21.7	0.004	200	1.3	6.1
SM-489	27.2	0.007	200	2.0	7.3	27.2	0.007	200	2.0	7.3
SM-490	24.9	0.006	200	4.6	18.6	24.9	0.006	200	4.7	18.7
SM-491	26.7	0.007	200	2.0	7.4	26.7	0.007	200	2.0	7.5
SM-492	22.4	0.005	200	1.3	5.9	22.4	0.005	200	1.3	6.0
SM-493	21.6	0.004	200	0.7	3.1	21.6	0.004	200	0.7	3.1
SM-496	22.8	0.005	200	7.3	31.9	22.8	0.005	200	7.3	32.1
SM-497	106.1	0.105	200	7.9	7.5	106.1	0.105	200	8.0	7.5
SM-498	24.5	0.006	200	6.6	27.0	24.5	0.006	200	6.7	27.2
SM-499	27.2	0.007	200	3.3	12.1	27.2	0.007	200	3.3	12.2

Park St. SPS Results

		E	xisting Condition	ns		Future Co	onditions - Rus	sell/Millennium S	Base Case	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-500	39.1	0.004	250	30.4	77.8	39.1	0.004	250	30.6	78.4
SM-501	38.0	0.013	200	0.7	1.7	38.0	0.013	200	0.7	1.8
SM-502	49.9	0.023	200	1.3	2.6	49.9	0.023	200	1.3	2.7
SM-503	26.2	0.006	200	5.3	20.2	26.2	0.006	200	5.7	21.7
SM-504	24.4	0.006	200	5.9	24.4	24.4	0.006	200	6.3	26.0
SM-509	39.2	0.014	200	0.7	1.7	39.2	0.014	200	0.7	1.7
SM-510	32.9	0.01	200	1.3	4.0	32.9	0.01	200	1.3	4.0
SM-511	32.9	0.01	200	2.0	6.0	32.9	0.01	200	2.0	6.1
SM-512	26.6	0.007	200	3.3	12.4	26.6	0.007	200	3.7	13.8
SM-513	21.0	0.004	200	0.7	3.2	21.0	0.004	200	1.0	4.9
SM-515	29.0	0.008	200	2.6	9.1	29.0	0.008	200	2.7	9.2
SM-516	38.9	0.004	250	25.1	64.5	38.9	0.004	250	25.3	65.0
SM-518	64.5	0.012	250	14.5	22.6	64.5	0.012	250	14.7	22.7
SM-519	42.1	0.005	250	15.2	36.1	42.1	0.005	250	15.3	36.4
SM-520	36.8	0.004	250	24.5	66.5	36.8	0.004	250	24.6	67.0
SM-521	31.5	0.009	200	0.7	2.1	31.5	0.009	200	0.7	2.1
SM-522	32.6	0.01	200	2.0	6.1	32.6	0.01	200	2.0	6.1
SM-523	35.9	0.012	200	2.6	7.4	35.9	0.012	200	2.7	7.4
SM-524	36.3	0.012	200	3.3	9.1	36.3	0.012	200	3.3	9.2
SM-525	40.7	0.015	200	0.7	1.6	40.7	0.015	200	0.7	1.6
SM-526	39.1	0.014	200	0.7	1.7	39.1	0.014	200	0.7	1.7
SM-527	35.9	0.012	200	1.3	3.7	35.9	0.012	200	1.3	3.7
SM-528	26.4	0.006	200	2.0	7.5	26.4	0.006	200	2.0	7.6
SM-529	26.0	0.006	200	2.6	10.2	26.0	0.006	200	2.7	10.2
SM-530	25.7	0.006	200	3.3	12.9	25.7	0.006	200	3.3	13.0
SM-531	24.4	0.006	200	7.3	29.8	24.4	0.006	200	7.3	30.1
SM-532	24.4	0.006	200	7.9	32.5	24.4	0.006	200	8.0	32.7
SM-533	26.2	0.006	200	8.6	32.8	26.2	0.006	200	8.7	33.0
SM-534	34.1	0.011	200	0.7	1.9	34.1	0.011	200	0.7	2.0
SM-535	37.6	0.013	200	1.3	3.5	37.6	0.013	200	1.3	3.5
SM-536	26.5	0.007	200	2.0	7.5	26.5	0.007	200	2.0	7.5
SM-537	28.6	0.008	200	5.9	20.8	28.6	0.008	200	6.0	21.0
SM-538	25.8	0.006	200	6.6	25.6	25.8	0.006	200	6.7	25.8
SM-539	23.1	0.005	200	7.9	34.4	23.1	0.005	200	8.0	34.6
SM-540	46.1	0.02	200	0.7	1.4	46.1	0.02	200	0.7	1.4
SM-541	33.8	0.011	200	0.7	2.0	33.8	0.011	200	0.7	2.0
SM-542	37.1	0.004	250	42.3	114.1	37.1	0.004	250	43.0	115.9
SM-543	153.5	0.005	400	43.0	28.0	153.5	0.005	400	43.6	28.4
SM-544	141.2	0.056	250	1.3	0.9	141.2	0.056	250	1.3	0.9

Park St. SPS Results

		Е	xisting Conditio	ns		Future Co	onditions - Russ	ell/Millennium S	Sewer Capacity I	Base Case
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-545	51.0	0.007	250	2.0	3.9	51.0	0.007	250	2.0	3.9
SM-546	50.6	0.007	250	2.6	5.2	50.6	0.007	250	2.7	5.3
SM-547	52.7	0.008	250	4.0	7.5	52.7	0.008	250	4.0	7.6
SM-548	46.8	0.006	250	4.6	9.9	46.8	0.006	250	4.7	10.0
SM-549	51.5	0.008	250	5.3	10.3	51.5	0.008	250	5.3	10.3
SM-550	41.3	0.005	250	13.2	32.0	41.3	0.005	250	13.3	32.2
SM-551	37.4	0.004	250	13.9	37.1	37.4	0.004	250	14.0	37.4
SM-552	31.5	0.003	250	1.3	4.2	31.5	0.003	250	1.3	4.2
SM-553	29.8	0.008	200	2.0	6.6	29.8	0.008	200	2.0	6.7
SM-554	39.6	0.015	200	3.3	8.3	39.6	0.015	200	3.3	8.4
SM-555	26.6	0.007	200	4.0	14.9	26.6	0.007	200	4.0	15.0
SM-556	22.9	0.005	200	12.6	54.8	22.9	0.005	200	12.7	55.2
SM-557	42.9	0.005	250	25.8	60.2	42.9	0.005	250	26.0	60.6
SM-558	75.4	0.016	250	26.4	35.1	75.4	0.016	250	26.6	35.3
SM-559	39.3	0.014	200	0.7	1.7	39.3	0.014	200	0.7	1.7
SM-560	89.4	0.074	200	2.0	2.2	89.4	0.074	200	2.0	2.2
SM-561	36.1	0.012	200	2.6	7.3	36.1	0.012	200	2.7	7.4
SM-562	23.6	0.005	200	3.3	14.0	23.6	0.005	200	3.3	14.1
SM-563	37.4	0.013	200	0.0	0.0	37.4	0.013	200	0.0	0.0
SM-578	20.6	0.004	200	0.7	3.2	20.6	0.004	200	0.7	3.2
SM-580	31.6	0.003	250	1.3	4.2	31.6	0.003	250	1.3	4.2
SM-581	26.9	0.002	250	2.0	7.4	26.9	0.002	250	2.0	7.4
SM-582	36.8	0.004	250	3.3	9.0	36.8	0.004	250	3.3	9.0
SM-583	30.0	0.003	250	4.0	13.2	30.0	0.003	250	4.0	13.3
SM-584	30.0	0.003	250	5.3	17.6	30.0	0.003	250	5.3	17.8
SM-585	26.4	0.002	250	5.9	22.5	26.4	0.002	250	6.0	22.7
SM-607	31.4	0.003	250	0.0	0.0	31.4	0.003	250	0.0	0.0
SM-608	31.6	0.003	250	0.7	2.1	31.6	0.003	250	0.7	2.1
SM-613	47.1	0.006	250	1.3	2.8	47.1	0.006	250	1.3	2.8
SM-803	27.8	0.002	250	4.6	16.7	27.8	0.002	250	4.7	16.8
SM-804	23.4	0.002	250	2.6	11.3	23.4	0.002	250	2.7	11.4
SM-847	43.7	0.018	200	0.7	1.5	43.7	0.018	200	0.7	1.5
SM-980	48.5	0.007	250	3.3	6.8	48.5	0.007	250	3.3	6.9

Notes:

Denotes greater than 80% capacity utilized.

Denotes greater than 100% capacity utilized (i.e. surcharging).

		E	xisting Conditio	ns			F	uture Condition	าร	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SAN 1	33.5	0.003	250	0.2	0.7	33.5	0.003	250	0.5	1.6
SAN 2	32.5	0.003	250	5.9	18.0	32.5	0.003	250	12.6	38.8
SAN 3	24.0	0.002	250	6.1	25.4	24.0	0.002	250	13.2	54.8
SAN 4	23.2	0.005	200	5.1	22.1	23.2	0.005	200	11.0	47.6
SAN 193	32.7	0.003	250	0.5	1.5	32.7	0.003	250	1.1	3.2
SAN 194	31.4	0.003	250	11.7	37.3	31.4	0.003	250	25.2	80.5
SAN 195	26.9	0.002	250	6.3	23.6	26.9	0.002	250	13.7	50.8
SAN 278	39.4	0.014	200	4.9	12.4	39.4	0.014	200	10.5	26.7
SAN 279	18.7	0.003	200	2.7	14.4	18.7	0.003	200	5.8	31.0
SAN 280	21.0	0.004	200	2.4	11.6	21.0	0.004	200	5.3	25.1
SAN 281	32.0	0.010	200	0.2	0.8	32.0	0.010	200	0.5	1.6
SAN 282	23.5	0.005	200	0.2	1.0	23.5	0.005	200	0.5	2.2
SAN 283	21.1	0.004	200	1.5	6.9	21.1	0.004	200	3.2	14.9
SAN 284	19.7	0.004	200	0.7	3.7	19.7	0.004	200	1.6	8.0
SAN 285	20.3	0.004	200	0.5	2.4	20.3	0.004	200	1.1	5.2
SAN 286	23.0	0.005	200	0.2	1.1	23.0	0.005	200	0.5	2.3
SAN 287	23.4	0.005	200	0.2	1.0	23.4	0.005	200	0.5	2.3
SAN 288	20.7	0.004	200	0.5	2.4	20.7	0.004	200	1.1	5.1
SAN 289	20.6	0.004	200	0.7	3.6	20.6	0.004	200	1.6	7.7
SAN 290	19.1	0.003	200	1.0	5.1	19.1	0.003	200	2.1	11.0
SAN 291	20.0	0.004	200	1.2	6.1	20.0	0.004	200	2.6	13.2
SAN 292	20.0	0.004	200	1.5	7.3	20.0	0.004	200	3.2	15.8
SAN 293	19.7	0.004	200	1.0	4.9	19.7	0.004	200	2.1	10.7
SAN 294	19.2	0.003	200	1.2	6.4	19.2	0.003	200	2.6	13.7
SAN 295	25.0	0.006	200	1.7	6.8	25.0	0.006	200	3.7	14.7
SAN 296	21.2	0.004	200	1.7	8.1	21.2	0.004	200	3.7	17.4
SAN 297	20.2	0.004	200	2.0	9.7	20.2	0.004	200	4.2	20.9
SAN 298	21.3	0.004	200	2.2	10.3	21.3	0.004	200	4.7	22.2
SAN 299	22.6	0.005	200	3.4	15.1	22.6	0.005	200	7.4	32.6
SAN 300	21.0	0.004	200	0.7	3.5	21.0	0.004	200	1.6	7.5
SAN 301	21.0	0.004	200	1.0	4.6	21.0	0.004	200	2.1	10.0
SAN 302	37.5	0.013	200	1.2	3.3	37.5	0.013	200	2.6	7.0
SAN 303	23.9	0.005	200	0.5	2.0	23.9	0.005	200	1.1	4.4
SAN 304	50.8	0.024	200	0.2	0.5	50.8	0.024	200	0.5	1.0
SAN 305	25.8	0.006	200	0.5	1.9	25.8	0.006	200	1.1	4.1
SAN 306	27.5	0.007	200	0.7	2.7	27.5	0.007	200	1.6	5.7
SAN 307	34.3	0.011	200	1.0	2.8	34.3	0.011	200	2.1	6.1
SAN 311	31.9	0.009	200	2.2	6.9	31.9	0.009	200	4.7	14.8

		E	xisting Conditio	ns			F	uture Condition	ns	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SAN 312	34.7	0.011	200	2.4	7.0	34.7	0.011	200	5.3	15.2
SAN 313	46.6	0.020	200	2.7	5.8	46.6	0.020	200	5.8	12.4
SAN 314	20.8	0.004	200	4.9	23.5	20.8	0.004	200	10.5	50.6
SAN 315	37.5	0.004	250	1.2	3.3	37.5	0.004	250	2.6	7.0
SAN 317	57.9	0.009	250	1.5	2.5	57.9	0.009	250	3.2	5.4
SAN 321	49.3	0.023	200	1.0	2.0	49.3	0.023	200	2.1	4.3
SAN 322	25.7	0.006	200	1.2	4.7	25.7	0.006	200	2.6	10.2
SAN 323	20.9	0.004	200	1.5	7.0	20.9	0.004	200	3.2	15.1
SAN 324	20.8	0.004	200	0.5	2.4	20.8	0.004	200	1.1	5.1
SAN 325	32.7	0.010	200	0.2	0.7	32.7	0.010	200	0.5	1.6
SAN 326	46.2	0.020	200	0.2	0.5	46.2	0.020	200	0.5	1.1
SAN 327	20.9	0.004	200	2.0	9.3	20.9	0.004	200	4.2	20.1
SAN 328	85.2	0.021	250	1.7	2.0	85.2	0.021	250	3.7	4.3
SAN 329	74.4	0.016	250	2.0	2.6	74.4	0.016	250	4.2	5.7
SAN 330	28.7	0.002	250	12.4	43.4	28.7	0.002	250	26.8	93.6
SAN 331	32.7	0.003	250	12.2	37.4	32.7	0.003	250	26.3	80.5
SAN 332	31.6	0.003	250	12.0	37.8	31.6	0.003	250	25.8	81.6
SAN 333	47.2	0.006	250	6.6	14.0	47.2	0.006	250	14.2	30.1
SAN 334	20.8	0.004	200	0.2	1.2	20.8	0.004	200	0.5	2.5
SAN 341	29.4	0.008	200	0.2	0.8	29.4	0.008	200	0.5	1.8
SAN 342	27.4	0.007	200	2.0	7.1	27.4	0.007	200	4.2	15.4
SM-81	48.9	0.022	200	0.2	0.5	48.9	0.022	200	0.5	1.1
SM-82	19.9	0.004	200	0.5	2.4	19.9	0.004	200	1.1	5.3
SM-83	21.7	0.004	200	0.7	3.4	21.7	0.004	200	1.6	7.3
SM-268	21.5	0.004	200	0.5	2.3	21.5	0.004	200	1.1	4.9
SM-269	21.7	0.004	200	0.2	1.1	21.7	0.004	200	0.5	2.4
SM-271	24.0	0.005	200	0.2	1.0	24.0	0.005	200	0.5	2.2
SM-272	25.7	0.006	200	0.5	1.9	25.7	0.006	200	1.1	4.1
SM-274	18.4	0.003	200	0.7	4.0	18.4	0.003	200	1.6	8.6
SM-276	33.5	0.003	250	1.0	2.9	33.5	0.003	250	2.1	6.3
SM-277	34.3	0.003	250	1.2	3.6	34.3	0.003	250	2.6	7.7
SM-278	35.4	0.004	250	1.5	4.1	35.4	0.004	250	3.2	8.9
SM-279	31.2	0.003	250	1.7	5.5	31.2	0.003	250	3.7	11.8
SM-280	35.5	0.004	250	2.0	5.5	35.5	0.004	250	4.2	11.8
SM-281	215.8	0.006	450	27.8	12.9	215.8	0.006	450	60.0	27.8
SM-282	161.0	0.003	450	28.1	17.4	161.0	0.003	450	60.5	37.6
SM-283	146.6	0.003	450	28.3	19.3	146.6	0.003	450	61.0	41.6
SM-284	149.5	0.003	450	28.5	19.1	149.5	0.003	450	61.5	41.2

		E	xisting Conditio	ns			F	uture Condition	าร	
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-285	137.1	0.002	450	28.8	21.0	137.1	0.002	450	62.1	45.3
SM-286	146.2	0.003	450	29.0	19.9	146.2	0.003	450	62.6	42.8
SM-294	148.1	0.007	375	14.6	9.9	148.1	0.007	375	31.6	21.3
SM-295	75.1	0.002	375	14.9	19.8	75.1	0.002	375	32.1	42.7
SM-296	138.1	0.006	375	15.1	11.0	138.1	0.006	375	32.6	23.6
SM-297	93.1	0.003	375	18.3	19.7	93.1	0.003	375	39.5	42.4
SM-298	126.8	0.005	375	18.5	14.6	126.8	0.005	375	40.0	31.5
SM-299	123.9	0.005	375	19.8	15.9	123.9	0.005	375	42.6	34.4
SM-300	246.0	0.020	375	25.4	10.3	246.0	0.020	375	54.7	22.2
SM-301	459.5	0.069	375	25.6	5.6	459.5	0.069	375	55.2	12.0
SM-302	21.9	0.004	200	2.2	10.0	21.9	0.004	200	4.7	21.7
SM-303	21.5	0.004	200	2.4	11.3	21.5	0.004	200	5.3	24.5
SM-304	21.6	0.004	200	2.7	12.4	21.6	0.004	200	5.8	26.7
SM-305	19.8	0.004	200	2.9	14.8	19.8	0.004	200	6.3	31.8
SM-306	45.5	0.019	200	2.0	4.3	45.5	0.019	200	4.2	9.3
SM-307	46.0	0.020	200	2.2	4.8	46.0	0.020	200	4.7	10.3
SM-308	55.2	0.028	200	2.4	4.4	55.2	0.028	200	5.3	9.5
SM-309	23.9	0.005	200	0.2	1.0	23.9	0.005	200	0.5	2.2
SM-310	39.8	0.004	250	0.2	0.6	39.8	0.004	250	0.5	1.3
SM-311	42.0	0.005	250	0.7	1.7	42.0	0.005	250	1.6	3.8
SM-312	167.6	0.079	250	1.0	0.6	167.6	0.079	250	2.1	1.3
SM-313	55.9	0.029	200	0.2	0.4	55.9	0.029	200	0.5	0.9
SM-314	48.6	0.022	200	1.0	2.0	48.6	0.022	200	2.1	4.3
SM-315	47.1	0.021	200	1.2	2.6	47.1	0.021	200	2.6	5.6
SM-316	21.1	0.004	200	0.2	1.2	21.1	0.004	200	0.5	2.5
SM-317	20.8	0.004	200	0.5	2.3	20.8	0.004	200	1.1	5.0
SM-327	21.3	0.004	200	0.2	1.1	21.3	0.004	200	0.5	2.5
SM-328	20.2	0.004	200	0.5	2.4	20.2	0.004	200	1.1	5.2
SM-329	20.1	0.004	200	0.7	3.6	20.1	0.004	200	1.6	7.8
SM-431	23.5	0.005	200	1.0	4.2	23.5	0.005	200	2.1	9.0
SM-570	21.5	0.004	200	1.2	5.7	21.5	0.004	200	2.6	12.3
SM-797	44.0	0.001	375	18.1	41.0	44.0	0.001	375	38.9	88.4
SM-911	47.4	0.021	200	0.2	0.5	47.4	0.021	200	0.5	1.1
SM-912	42.1	0.016	200	0.5	1.2	42.1	0.016	200	1.1	2.5
SM-913	25.8	0.006	200	0.7	2.8	25.8	0.006	200	1.6	6.1
SM-914	22.4	0.005	200	1.0	4.3	22.4	0.005	200	2.1	9.4
SM-915	33.3	0.010	200	1.2	3.7	33.3	0.010	200	2.6	7.9
SM-916	43.0	0.017	200	1.2	2.8	43.0	0.017	200	2.6	6.1

Existing Conditions							Future Conditions				
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	
SM-917	46.8	0.020	200	1.0	2.1	46.8	0.020	200	2.1	4.5	
SM-918	43.3	0.017	200	0.7	1.7	43.3	0.017	200	1.6	3.6	
SM-919	36.7	0.013	200	0.5	1.3	36.7	0.013	200	1.1	2.9	
SM-920	20.8	0.004	200	0.2	1.2	20.8	0.004	200	0.5	2.5	
SM-921	13.3	0.002	200	2.7	20.2	13.3	0.002	200	5.8	43.5	

Notes:

Denotes greater than 80% capacity utilized.
Denotes greater than 100% capacity utilized (i.e. surcharging).

Kincardine SPS Results

		E	xisting Condition	ns		Futi	ure Conditions -	Brown to 832 a	nd Brigadoon to	380
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-18	20.7	0.004	200	0.4	1.8	20.7	0.004	200	0.4	1.8
SM-19	20.6	0.004	200	8.0	3.7	20.6	0.004	200	8.0	3.7
SM-20	30.1	0.008	200	0.0	0.0	30.1	0.008	200	0.0	0.0
SM-21	21.8	0.004	200	0.4	1.7	21.8	0.004	200	0.4	1.8
SM-22	21.3	0.004	200	8.0	3.6	21.3	0.004	200	8.0	3.6
SM-23	34.8	0.011	200	1.1	3.3	34.8	0.011	200	1.1	3.3
SM-24	20.5	0.004	200	0.4	1.8	20.5	0.004	200	0.4	1.9
SM-26	48.9	0.022	200	1.9	3.9	48.9	0.022	200	1.9	3.9
SM-27	21.5	0.004	200	0.4	1.8	21.5	0.004	200	0.4	1.8
SM-28	20.6	0.004	200	0.8	3.7	20.6	0.004	200	0.8	3.7
SM-29	20.7	0.004	200	1.1	5.5	20.7	0.004	200	1.1	5.5
SM-30	31.6	0.009	200	0.4	1.2	31.6	0.009	200	0.4	1.2
SM-31	23.1	0.005	200	0.8	3.3	23.1	0.005	200	0.8	3.3
SM-32	21.6	0.004	200	1.1	5.3	21.6	0.004	200	1.1	5.3
SM-33	24.8	0.006	200	0.0	0.0	24.8	0.006	200	0.0	0.0
SM-34	13.7	0.002	200	0.4	2.8	13.7	0.002	200	0.4	2.8
SM-35	20.6	0.004	200	0.8	3.7	20.6	0.004	200	0.8	3.7
SM-36	58.4	0.032	200	1.1	1.9	58.4	0.032	200	1.1	2.0
SM-37	18.8	0.003	200	2.3	12.1	18.8	0.003	200	2.3	12.2
SM-38	19.2	0.003	200	2.7	13.8	19.2	0.003	200	2.7	13.9
SM-39	30.1	0.003	250	3.4	11.3	30.1	0.003	250	3.4	11.4
SM-40	31.9	0.003	250	3.8	11.9	31.9	0.003	250	3.8	12.0
SM-41	33.5	0.003	250	4.2	12.5	33.5	0.003	250	4.2	12.6
SM-42	30.4	0.003	250	4.5	15.0	30.4	0.003	250	4.6	15.1
SM-43	56.1	0.003	300	4.5	8.1	56.1	0.003	300	4.6	8.2
SM-44	49.4	0.003	300	4.9	10.0	49.4	0.003	300	5.0	10.1
SM-45	50.8	0.003	300	9.9	19.4	50.8	0.003	300	9.9	19.6
SM-46	50.7	0.003	300	10.2	20.2	50.7	0.003	300	10.3	20.4
SM-47	20.9	0.004	200	1.9	9.1	20.9	0.004	200	1.9	9.1
SM-48	21.8	0.004	200	2.3	10.5	21.8	0.004	200	2.3	10.5
SM-49	19.2	0.003	200	2.7	13.8	19.2	0.003	200	2.7	13.9
SM-50	21.9	0.004	200	4.9	22.5	21.9	0.004	200	5.0	22.7
SM-51	33.1	0.01	200	5.3	16.1	33.1	0.010	200	5.3	16.2
SM-52	23.2	0.005	200	1.5	6.5	23.2	0.005	200	1.5	6.6
SM-53	19.5	0.004	200	0.4	1.9	19.5	0.004	200	0.4	2.0
SM-54	20.9	0.004	200	1.1	5.4	20.9	0.004	200	1.1	5.5
SM-55	20.8	0.004	200	0.8	3.7	20.8	0.004	200	0.8	3.7
SM-56	23.4	0.005	200	0.4	1.6	23.4	0.005	200	0.4	1.6
SM-57	42.6	0.017	200	0.8	1.8	42.6	0.017	200	0.8	1.8

Kincardine SPS Results

		E	xisting Conditio	ns		Fut	ure Conditions -	Brown to 832 a	nd Brigadoon t	o 380
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-58	24.1	0.005	200	1.9	7.9	24.1	0.005	200	1.9	7.9
SM-59	20.7	0.004	200	0.4	1.8	20.7	0.004	200	0.4	1.8
SM-60	20.6	0.004	200	0.8	3.7	20.6	0.004	200	0.8	3.7
SM-61	32.5	0.01	200	0.4	1.2	32.5	0.010	200	0.4	1.2
SM-62	27.6	0.007	200	1.5	5.5	27.6	0.007	200	1.5	5.5
SM-63	27.4	0.007	200	1.9	6.9	27.4	0.007	200	1.9	7.0
SM-64	24.1	0.005	200	2.3	9.4	24.1	0.005	200	2.3	9.5
SM-65	23.3	0.005	200	2.7	11.4	23.3	0.005	200	2.7	11.5
SM-67	20.8	0.004	200	3.4	16.4	20.8	0.004	200	3.4	16.6
SM-68	29.0	0.008	200	6.1	20.9	29.0	0.008	200	6.1	21.1
SM-69	30.3	0.009	200	6.4	21.2	30.3	0.009	200	6.5	21.4
SM-72	32.4	0.01	200	0.4	1.2	32.4	0.010	200	0.4	1.2
SM-73	28.6	0.008	200	0.8	2.6	28.6	0.008	200	0.8	2.7
SM-74	35.5	0.012	200	0.4	1.1	35.5	0.012	200	0.4	1.1
SM-75	26.1	0.006	200	0.8	2.9	26.1	0.006	200	0.8	2.9
SM-76	25.9	0.006	200	1.1	4.4	25.9	0.006	200	1.1	4.4
SM-77	33.7	0.011	200	0.4	1.1	33.7	0.011	200	0.4	1.1
SM-78	23.9	0.005	200	1.9	7.9	23.9	0.005	200	1.9	8.0
SM-79	24.6	0.006	200	2.3	9.3	24.6	0.006	200	2.3	9.3
SM-80	46.5	0.02	200	0.4	0.8	46.5	0.020	200	0.4	0.8
SM-84	132.3	0.006	375	23.1	17.5	132.3	0.006	375	56.4	42.6
SM-86	70.2	0.014	250	0.4	0.5	70.2	0.014	250	22.7	32.3
SM-88	80.0	0.018	250	1.1	1.4	80.0	0.018	250	23.5	29.3
SM-89	78.1	0.017	250	8.0	10.2	78.1	0.017	250	30.3	38.9
SM-90	41.4	0.005	250	8.3	20.1	41.4	0.005	250	30.7	74.2
SM-92	91.5	0.003	375	23.5	25.7	91.5	0.003	375	56.8	62.0
SM-98	102.9	0.011	300	15.9	15.5	102.9	0.011	300	16.0	15.6
SM-99	188.8	0.038	300	23.9	12.6	188.8	0.038	300	57.1	30.3
SM-576	81.2	0.019	250	0.8	0.9	81.2	0.019	250	23.1	28.4
SM-586	20.4	0.004	200	0.4	1.9	20.4	0.004	200	0.4	1.9
SM-587	20.7	0.004	200	1.9	9.1	20.7	0.004	200	1.9	9.2
SM-588	20.8	0.004	200	2.3	10.9	20.8	0.004	200	2.3	11.0
SM-589	20.7	0.004	200	2.7	12.8	20.7	0.004	200	2.7	12.9
SM-590	20.8	0.004	200	3.0	14.6	20.8	0.004	200	3.1	14.7
SM-591	20.8	0.004	200	8.3	40.1	20.8	0.004	200	8.4	40.4
SM-592	20.8	0.004	200	8.7	41.8	20.8	0.004	200	8.8	42.2
SM-593	21.1	0.004	200	11.4	53.8	21.1	0.004	200	22.2	105.1
SM-594	11.1	0.001	200	11.7	105.5	11.1	0.001	200	22.6	202.9
SM-595	49.9	0.023	200	13.6	27.3	49.9	0.023	200	24.5	49.1

Kincardine SPS Results

Existing Conditions						Futi	Future Conditions - Brown to 832 and Brigadoon to 380				
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	
SM-596	58.7	0.032	200	14.0	23.9	58.7	0.032	200	24.9	42.4	
SM-597	43.9	0.018	200	14.4	32.8	43.9	0.018	200	25.3	57.6	
SM-599	88.7	0.073	200	1.1	1.3	88.7	0.073	200	1.1	1.3	
SM-810	20.5	0.004	200	1.5	7.4	20.5	0.004	200	1.5	7.4	
SM-811	16.6	0.003	200	1.1	6.8	16.6	0.003	200	1.1	6.9	
SM-812	31.1	0.009	200	0.8	2.4	31.1	0.009	200	0.8	2.5	
SM-813	46.7	0.02	200	0.4	0.8	46.7	0.020	200	0.4	0.8	
SM-814	20.8	0.004	200	2.3	10.9	20.8	0.004	200	13.0	62.6	
SM-815	23.8	0.005	200	0.4	1.6	23.8	0.005	200	0.4	1.6	
SM-816	21.0	0.004	200	0.8	3.6	21.0	0.004	200	0.8	3.6	
SM-817	18.5	0.003	200	1.1	6.2	18.5	0.003	200	1.1	6.2	
SM-818	26.1	0.006	200	1.5	5.8	26.1	0.006	200	12.3	47.1	
SM-819	20.7	0.004	200	1.9	9.2	20.7	0.004	200	12.7	61.3	
SM-834	17.1	0.013	150	0.4	2.2	17.1	0.013	150	0.4	2.2	
SM-835	18.8	0.015	150	0.8	4.0	18.8	0.015	150	0.8	4.1	
SM-841	26.6	0.007	200	3.0	11.4	26.6	0.007	200	3.1	11.5	
SM-963	29.7	0.008	200	4.9	16.6	29.7	0.008	200	5.0	16.7	
SM-964	21.6	0.004	200	4.5	21.1	21.6	0.004	200	4.6	21.2	
SM-965	25.8	0.006	200	2.7	10.3	25.8	0.006	200	2.7	10.4	
SM-966	21.2	0.004	200	2.3	10.7	21.2	0.004	200	2.3	10.8	
SM-967	21.9	0.004	200	0.4	1.7	21.9	0.004	200	0.4	1.7	
SM-969	45.6	0.019	200	0.4	0.8	45.6	0.019	200	0.4	8.0	
SM-970	45.6	0.019	200	0.8	1.7	45.6	0.019	200	0.8	1.7	
SM-971	45.0	0.019	200	1.1	2.5	45.0	0.019	200	1.1	2.5	
SM-972	43.6	0.018	200	1.5	3.5	43.6	0.018	200	1.5	3.5	
SM-973	42.7	0.017	200	0.4	0.9	42.7	0.017	200	0.4	0.9	
SM-974	39.5	0.015	200	0.8	1.9	39.5	0.015	200	0.8	1.9	
SM-975	38.8	0.014	200	1.1	2.9	38.8	0.014	200	1.1	3.0	
SM-976	39.7	0.015	200	1.5	3.8	39.7	0.015	200	1.5	3.8	

Notes:

Denotes greater than 80% capacity utilized.

Denotes greater than 100% capacity utilized (i.e. surcharging).

Appendix F Tiverton SewerCAD® Modelling Information

Municipality of Kincardine Tiverton Area SPS Catchments Flow Notes for Master Plan

Job # : 22128

Date : November 16, 2022

Revised : November 30, 2022

1.0 Background

The Municipality of Kincardine is updating its Water & Wastewater Master Plan to evaluate water and wastewater servicing needs for Kincardine, Tiverton, and the Lakeshore Area. The original Master Plan was completed under BMROSS File No. 16130.

The purpose of these notes is to summarize catchment area and design flow information for the King Street SPS (Secondary) and Maple Street SPS (Main) in Tiverton.

2.0 Design Data

Reference	<u>Item</u>		
22128 Nov. 4 Reserve	Persons per household	2.48	p/ERU
Capacity Draft Memo	Number of customers for entire BEC wastewater system	460	customers
9 June 2022 Sewer Users Table	Number of customers for Tiverton Sewers	370	customers
This Sheet	Number of ERUs in Tiverton SPS Catchment Area	354	ERUs
2019-2021 Annual Reports for	Average day flow to BEC Lagoons	730	m³/day
BEC Lagoon System	Average day flow from Tiverton	342	m³/day
	l Per Tiverton sewer customer wastwater flow (all year avg.) Per-ERU average day flow		L/cust/day L/ERU/day
22128 Nov. 22 Development Commitments Table	Units - Tiverton-wide	30	ERUs
MECP Guidelines	I/I allowance Industrial flow allowance Commercial/Institutional flow allowance	0.405	L/ha/s L/ha/s L/ha/s

3.0 King Street SPS (BMROSS 74026 Briar Hill SPS)

The existing areas and residential units remain unchanged since the last Master Plan. Flow allowances per hectare and ERU have been updated.

Future flow is based on the Conquergood development (109 ERUs, 5.44 ha) and a proportionate amount of infill (4.8 ERUs).

Item Residential Area Industrial Area Commercial/Institutional Area ERUs from Residential ERUs from Residential as percent of Tiverton	Existing 6.42 0 0.82 58 16%	Future 11.86 0 0.82 172	Units ha ha ha units
Calculate Residential Population Peaking factor Average day residential flow Average day industrial flow Average day commercial/institutional flow Average day flow; total	144 4.20 0.62 0.00 0.27 0.9	426 4.01 1.83 0.00 0.27 2.1	people - L/s L/s L/s L/s
I/I allowance	2.02720	3.550400	L/s
Peak instantaneous flow; total incl. I/I	5.74384	11.969197	L/s

4.0 <u>Maple Street SPS (BMROSS 74026 Main SPS)</u>

The existing areas and residential units remain unchanged since the last Master Plan. Flow allowances per hectare and ERU have been updated. The Maple St. SPS accepts discharge from the King St. SPS.

Future flow is based on the Kaydan Drive (14 ERUs, 0.92 ha), Pine Tree Camp (59 ERUs, 8.16 ha), Rae Street (25 ERUs, 1.49 ha), Maple Street (14 ERUs, 0.82 ha) and Mackwade (5 ERUs, 0.36 ha) developments, and a proportionate amount of infill (25.2 ERUs)

4.1 Maple Street SPS Catchment Area

<u>Item</u>	<u>Existing</u>	<u>Future</u>	<u>Units</u>					
Residential Area	50.19	61.96	ha					
Industrial Area	0	0	ha					
Commercial/Institutional Area	13.31	13.31	ha					
ERUs from Residential	296	438	ERUs					
ERUs from Residential as percent of Tiverton	84%							
Calculate								
Residential Population	734	1086	people					
Peaking factor	3.88	3.78						
Average day residential flow	3.16	4.68	L/s					
Average day industrial flow	0.00	0.00	L/s					
Average day commercial/institutional flow	4.31	4.31	L/s					
Average day flow; total	7.5	9.0	L/s					
I/I allowance	17.780000	21.074873	L/s					
Peak instantaneous flow; total incl. I/I	46.806138	55.039445	L/s					
Maple Street SPS Total Flow - Catchment Area + King Street SPS Flows								

4.2

52.549977 67.008642 L/s Peak instantaneous flow; total incl. I/I

Municipality of Kincardine SewerCAD Modelling for Master Plan Tiverton - Calculations and Notes

Job # : 22128

Date : November 24, 2022

Revised : November 25, 2022

1.0 Background

The Municipality of Kincardine is completing a water and wastewater Master Plan process. The sewage servicing component will include a review of servicing existing development and future development. The purpose of these notes is to summarize data used to create a SewerCAD model, and the results of that modelling.

1 22128 - Tiverton Area SPS Catchments: Flow Notes for Master Plan

1.1 References

1 <u>22128 - Kincardine Area SPS Catchments: Flow Notes for Master Plan</u>

2.0 Analysis & Model Data

2.1 Data

<u>F</u>	<u>Reference</u>	<u>Item</u>	
a.	1	King Street SPS	
		Ex. peak sewage flow Ex. I&I allowance Ex. total peak flow	3.7 L/s 2.0 L/s 5.7 L/s
		Fut. peak sewage flow Fut. I&I allowance Fut. total peak flow	8.4 L/s 3.6 L/s 12.0 L/s
b.	1	Maple Street SPS Catchment A	Area (w/o King Street SPSs)
		Ex. peak sewage flow Ex. I&I allowance Ex. total peak flow Fut. peak sewage flow Fut. I&I allowance Fut. total peak flow	29.0 L/s 17.8 L/s 46.8 L/s 34.0 L/s 21.1 L/s 55.0 L/s
		Maple Street SPS Catchment A	Area (with King Street SPSs)
		Ex. total peak flow	52.5 L/s
		Fut. total peak flow	67.0 L/s

2.2 Sewage Flows by Manhole

For the existing system model, sewage flows to each manhole are calculated by dividing total peak flow for the catchment area by the number of maintenance holes.

For future flows, the sewage flow that is additional to existing is assigned to specific manholes based on future service area location in relation to existing manholes.

Ref	ference	<u>Item</u>				
	1 1 1 1	Flow per ERU Industrial flow allowance Commercial flow allowance Institutional flow allowance I-I allowance	0.011 0.405 0.324 0.324 0.280	L/ERU/s L/ha/s L/ha/s L/ha/s L/ha/s		
a.	1	King Street SPS				
		Ex. No. of manholes in model Ex. Peak flow per manhole Ex. Peaking Factor Future Peaking Factor				
		Additional future peak flow from	6.2	L/s		
		Conquergood Res. ERUs ICI	4.7 0.0	L/s L/s L/s L/s	at	SMH-702, SMH-704, SMH-707, SMH-719
		Net: Infill and Peak Reduction Infill Res. ERUs	0.2	L/s L/s	at	All King MHs
		PF Reduction of existing flow Check:	-0.2 6.2	L/s		
b.		Maple Street SPS Catchment Ex. No. of manholes in model Ex. Peak flow per manhole	73	<i>g Street SPSs)</i> MHs L/s/MH		
		Ex. Peaking Factor Future Peaking Factor	3.883 3.777			
		For King, add to gravity sewer Existing:		//H-672) L/s		
		Additional future peak flow from	14.5	L/s		
		King St. SPS	6.2		at	SMH-672

Kaydan		0.8 L/s		Page 3 of 6
ray darr	Res. ERUs	0.6 L/s		
	ICI	0.0 L/s	at	SMH-643
	I-I	0.3 L/s		
		0.0 _/0		
Pine Tree Carr	qı	4.7 L/s		
	Res. ERUs	2.4 L/s		01411.700
	ICI	0.0 L/s	at	SMH-722
	I-I	2.3 L/s		
Rae Street		1.4 L/s		
	Res. ERUs	1.0 L/s		01411.000
	ICI	0.0 L/s	at	SMH-692
	I-I	0.4 L/s		
Maple Street		0.8 L/s		01411.004
•	Res. ERUs	0.6 L/s	. 4	SMH-634,
	ICI	0.0 L/s	at	SMH-636,
	I-I	0.2 L/s		SMH-637
Mackwade		0.3 L/s		
	Res. ERUs	0.2 L/s	. 4	01411.057
	ICI	0.0 L/s	at	SMH-657
	I-I	0.1 L/s		
Net: Infill and F	Peak Reduction	0.2 L/s		
Infill	Res. ERUs	1.0 L/s	-4	A II A A = :- I = A A I I =
			at	All Maple MHs
PF Reduction	of existing flow	-0.8 L/s		
Check:		14.5 L/s		

		E	xisting Conditio	ns		Future Conditions				
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
014.040	22.2	0.000	000	2.2	0.4	22.2	0.000	000	0.0	0.0
SM-642	26.2	0.006	200	0.6	2.4	26.2	0.006	200	0.9	3.3
SM-643	38.7	0.014	200	1.3	3.3	38.7	0.014	200	1.7	4.5
SM-644	31.3	0.009	200	0.6	2.0	31.3	0.009	200	0.9	2.8
SM-645	42.5	0.017	200	1.3	3.0	42.5	0.017	200	1.7	4.1
SM-646	49.2	0.023	200	0.6	1.3	49.2	0.023	200	0.9	1.8
SM-647	28.8	0.008	200	0.6	2.2	28.8	0.008	200	0.9	3.0
SM-648	23.6	0.005	200	0.6	2.7	23.6	0.005	200	0.9	3.7
SM-649	36.8	0.013	200	0.6	1.7	36.8	0.013	200	0.9	2.3
SM-650	24.9	0.006	200	0.6	2.6	24.9	0.006	200	0.9	3.5
SM-651	45.3	0.019	200	0.6	1.4	45.3	0.019	200	0.9	1.9
SM-652	22.4	0.005	200	0.6	2.9	22.4	0.005	200	0.9	3.9
SM-653	35.4	0.012	200	0.6	1.8	35.4	0.012	200	0.9	2.4
SM-654	41.6	0.016	200	0.6	1.5	41.6	0.016	200	0.9	2.1
SM-655	42.3	0.017	200	0.6	1.5	42.3	0.017	200	0.9	2.0
SM-656	58.7	0.032	200	0.6	1.1	58.7	0.032	200	0.9	1.5
SM-657	30.4	0.003	250	4.5	14.7	30.4	0.003	250	7.5	24.6
SM-658	30.7	0.003	250	9.6	31.3	30.7	0.003	250	14.4	46.9
SM-659	31.5	0.009	200	0.6	2.0	31.5	0.009	200	1.1	3.6
SM-660	35.3	0.012	200	1.3	3.6	35.3	0.012	200	2.0	5.6
SM-661	182.1	0.011	375	45.5	25.0	182.1	0.011	375	72.8	40.0
SM-662	82.6	0.007	300	15.4	18.6	82.6	0.007	300	22.5	27.2
SM-663	163.6	0.009	375	47.4	29.0	163.6	0.009	375	75.7	46.2
SM-664	30.6	0.003	250	0.6	2.1	30.6	0.003	250	1.1	3.7
SM-665	39.0	0.004	250	1.3	3.3	39.0	0.004	250	2.3	5.8
SM-666	299.1	0.029	375	50.6	16.9	299.1	0.029	375	80.0	26.7
SM-667	183.8	0.008	400	52.5	28.6	183.8	0.008	400	83.1	45.2
SM-668	41.3	0.016	200	2.6	6.2	41.3	0.016	200	3.5	8.4
SM-669	44.2	0.018	200	0.6	1.5	44.2	0.018	200	0.9	2.0
SM-670	36.4	0.012	200	1.3	3.5	36.4	0.012	200	1.7	4.8
SM-671	33.1	0.010	200	1.9	5.8	33.1	0.010	200	2.6	7.8
SM-672	49.7	0.003	300	14.7	29.7	49.7	0.003	300	21.6	43.5
SM-673	49.4	0.023	200	3.8	7.8	49.4	0.023	200	5.5	11.1

		E	xisting Conditio	ns		Future Conditions				
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-674	42.2	0.017	200	3.2	7.6	42.2	0.017	200	4.6	11.0
SM-675	29.1	0.008	200	2.6	8.8	29.1	0.008	200	3.8	12.9
SM-676	39.3	0.014	200	1.3	3.3	39.3	0.014	200	2.0	5.2
SM-677	32.3	0.010	200	3.2	9.9	32.3	0.010	200	4.3	13.4
SM-678	30.7	0.009	200	3.8	12.5	30.7	0.009	200	5.2	16.9
SM-679	32.6	0.010	200	4.5	13.8	32.6	0.010	200	6.1	18.6
SM-680	27.2	0.007	200	3.8	14.1	27.2	0.007	200	6.6	24.3
SM-681	50.6	0.024	200	1.9	3.8	50.6	0.024	200	2.6	5.1
SM-682	22.2	0.005	200	2.6	11.6	22.2	0.005	200	4.9	22.0
SM-683	65.8	0.040	200	0.5	0.8	65.8	0.040	200	0.6	1.0
SM-684	46.3	0.020	200	0.3	0.6	46.3	0.020	200	0.3	0.7
SM-685	22.9	0.005	200	0.3	1.2	22.9	0.005	200	1.9	8.1
SM-686	20.9	0.004	200	0.8	3.9	20.9	0.004	200	2.5	11.9
SM-687	24.6	0.006	200	0.3	1.1	24.6	0.006	200	1.9	7.6
SM-688	20.4	0.004	200	1.4	6.7	20.4	0.004	200	4.7	22.8
SM-689	23.5	0.005	200	2.2	9.3	23.5	0.005	200	5.6	23.8
SM-690	68.6	0.044	200	0.3	0.4	68.6	0.044	200	1.9	2.7
SM-691	21.0	0.004	200	2.7	13.0	21.0	0.004	200	7.8	37.0
SM-692	21.9	0.004	200	3.0	13.8	21.9	0.004	200	8.1	36.9
SM-693	21.8	0.004	200	3.3	15.0	21.8	0.004	200	8.4	38.4
SM-694	31.1	0.009	200	0.6	2.1	31.1	0.009	200	0.9	2.8
SM-695	21.6	0.004	200	0.6	3.0	21.6	0.004	200	0.9	4.0
SM-696	37.4	0.013	200	0.3	0.7	37.4	0.013	200	0.3	0.8
SM-697	67.9	0.043	200	0.5	0.8	67.9	0.043	200	0.6	0.9
SM-698	76.6	0.055	200	0.8	1.1	76.6	0.055	200	0.9	1.2
SM-699	65.6	0.040	200	1.4	2.1	65.6	0.040	200	1.6	2.4
SM-700	21.5	0.004	200	0.3	1.3	21.5	0.004	200	0.3	1.5
SM-701	71.5	0.014	250	8.9	12.5	71.5	0.014	250	16.3	22.8
SM-702	50.0	0.007	250	9.6	19.2	50.0	0.007	250	17.2	34.3
SM-703	69.2	0.014	250	10.2	14.8	69.2	0.014	250	18.0	26.0
SM-704	65.4	0.005	300	17.3	26.4	65.4	0.005	300	28.4	43.4
SM-705	59.5	0.004	300	17.9	30.1	59.5	0.004	300	29.2	49.1
SM-706	33.1	0.010	200	0.6	1.9	33.1	0.010	200	0.9	2.6
SM-707	31.5	0.009	200	1.3	4.1	31.5	0.009	200	1.7	5.5
SM-708	22.4	0.005	200	0.6	2.9	22.4	0.005	200	1.7	7.5
SM-709	22.2	0.005	200	1.3	5.8	22.2	0.005	200	2.6	11.5
SM-710	30.4	0.009	200	4.5	14.8	30.4	0.009	200	6.1	19.9
SM-711	39.1	0.014	200	5.1	13.1	39.1	0.014	200	6.9	17.7
SM-712	40.5	0.015	200	1.9	4.8	40.5	0.015	200	2.6	6.4

Tiverton Combined SPS Results

	Existing Conditions					Future Conditions				
Conduit ID	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)	Full-Flow Capacity (L/s)	Slope (m/m)	Diameter (mm)	Calculated Flow (L/s)	Flow/Capacity (%)
SM-713	37.5	0.013	200	2.6	6.8	37.5	0.013	200	3.5	9.2
SM-714	166.3	0.009	375	29.5	17.7	166.3	0.009	375	49.5	29.7
SM-715	119.0	0.005	375	26.9	22.6	119.0	0.005	375	41.3	34.7
SM-716	115.9	0.004	375	28.8	24.9	115.9	0.004	375	48.6	41.9
SM-717	63.0	0.004	300	8.3	13.2	63.0	0.004	300	11.2	17.9
SM-718	38.6	0.014	200	0.6	1.7	38.6	0.014	200	5.5	14.3
SM-719	25.1	0.006	200	1.3	5.1	25.1	0.006	200	6.4	25.5
SM-720	49.2	0.003	300	3.8	7.8	49.2	0.003	300	5.2	10.6
SM-721	87.5	0.008	300	5.1	5.9	87.5	0.008	300	6.9	7.9
SM-722	87.5	0.008	300	5.8	6.6	87.5	0.008	300	7.8	8.9
SM-723	84.3	0.008	300	7.7	9.1	84.3	0.008	300	10.4	12.3
SM-724	30.7	0.009	200	1.3	4.2	30.7	0.009	200	1.7	5.6
SM-727	24.2	0.005	200	3.2	13.2	24.2	0.005	200	5.8	23.8
SM-731	54.2	0.027	200	0.6	1.2	54.2	0.027	200	0.9	1.6
SM-732	37.2	0.013	200	1.3	3.4	37.2	0.013	200	1.7	4.7
SM-733	63.3	0.004	300	1.9	3.0	63.3	0.004	300	2.6	4.1
SM-761	60.6	0.034	200	0.3	0.5	60.6	0.034	200	0.3	0.5
SM-762	20.4	0.004	200	6.4	31.3	20.4	0.004	200	12.8	62.9
SM-763	23.1	0.005	200	7.0	30.4	23.1	0.005	200	13.7	59.3
SM-772	51.6	0.025	200	3.6	6.9	51.6	0.025	200	10.3	19.9
SM-773	35.8	0.004	250	3.8	10.7	35.8	0.004	250	10.6	29.6
SM-774	32.1	0.003	250	4.1	12.8	32.1	0.003	250	10.9	34.0
SM-776	27.8	0.002	250	5.8	20.7	27.8	0.002	250	12.8	46.0

Notes:

Denotes greater than 80% capacity utilized.

Denotes greater than 100% capacity utilized (i.e. surcharging).

Appendix G Consultation Materials

MUNICIPALITY OF KINCARDINE



WATER AND WASTEWATER SERVICING MASTER PLAN UPDATE

NOTICE OF COMMENCEMENT AND OPEN HOUSE

THE PROJECT: The Municipality of Kincardine is updating the Water and Wastewater Servicing Master Plan for the communities of Kincardine, Tiverton and Lakeshore area from Kincardine to Inverhuron. The Master Plan was adopted in 2018 and since that time there has been residential and non-residential growth, additional development proposals and new and upgraded infrastructure within the systems. The updated Master Plan will involve a review of existing water treatment, storage, and supply infrastructure, and sanitary sewage collection, pumping, and treatment infrastructure. This update is being completed concurrently with a Municipal Class Environmental Assessment (MCEA) for an expansion to the Kincardine Water Treatment Plant, which is already an anticipated need based on potential community growth in combination with the potential to add Bruce Power as a water customer. Information from the Master Plan will inform and be used in the MCEA.

The Master Plan will include a review of future infrastructure needs for a 20-year period as they relate to key components of the water and wastewater systems. Upon completion, the Master Plan will serve as a strategic document for the continued provision of water and wastewater services that will integrate with other major planning, development and financing plans and policies. The Master Plan will also identify recommended projects and outline estimated timelines for implementation.

THE ENVIRONMENTAL ASSESSMENT PROCESS: The Water and Wastewater Servicing Master Plan Update is being conducted in accordance with the requirements of the Municipal Class Environmental Assessment process, dated October 2000, as amended in 2007, under the Environmental Assessment Act. Master Plan studies incorporate Phases 1 & 2 of the MCEA process and also includes consultation with the general public, community stakeholders, government review agencies and First Nation and Métis communities.

PUBLIC INVOLVEMENT & OPEN HOUSE: Public consultation is a key component of this study. Initial feedback or comments are welcome at this time. As a part of the consultation component of this project, a public open house will be held:

Tuesday November 29, 2022 from 6 PM – 8 PM Kincardine Municipal Administration Centre, 1475 Concession 5 Kincardine ON

Any comments collected will be maintained on file for use during the project and may be included in project documentation. With the exception of personal information, all comments will become part of the public record.

For further information on this project, please contact the consulting engineers: B. M. Ross and Associates, 62 North Street, Goderich Ontario, N7A 2T4. Telephone (519) 524-2641. Fax (519) 524-4403. Attention: Lisa Courtney, Environmental Planner. E-mail: lcourtney@bmross.net

Adam Weishar, Director of Infrastructure and Development, Municipality of Kincardine This Notice issued November 2, 2022



B. M. ROSS AND ASSOCIATES LIMITED
Engineers and Planners
62 North Street, Goderich, ON N7A 2T4
p. (519) 524-2641 www.bmross.net

File No. 22128

Agency letter

RE: Municipality of Kincardine
Water and Wastewater Servicing Master Plan Update

The Municipality of Kincardine is updating the Water and Wastewater Servicing Master Plan for the communities of Kincardine, Tiverton and Lakeshore area from Kincardine to Inverhuron. The Master Plan was adopted in 2018 and since that time there has been residential and non-residential growth, additional development proposals and new and upgraded infrastructure within the systems. The updated Master Plan will involve a review of existing water treatment, storage, and supply infrastructure, and sanitary sewage collection, pumping, and treatment infrastructure. This update is being completed concurrently with a Municipal Class Environmental Assessment (MCEA) for an expansion to the Kincardine Water Treatment Plant, which is already an anticipated need based on potential community growth in combination with the potential to add Bruce Power as a water customer. Information from the Master Plan will inform and be used in the MCEA.

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This project is following the process for Master Plans under the MCEA. The proposed approach for this Master Plan (Approach 1) is a broad level assessment that will be the basis for and used in support of future investigations of specific projects. A Master Plan document will be prepared at the end of Phases 1 and 2 of the MCEA process. The study will include consultation with the public, First Nation and Métis communities, stakeholders and review agencies (see attached Public Notice). This letter is being issued to advise of the start of the Master Plan. There will be additional opportunities for input and involvement as the study progresses.

Your organization was identified as possibly having an interest in this project and we are soliciting your input. Please forward your response to the undersigned by November 23rd, 2022. If you have any questions or require further information, please contact the undersigned at lcourtney@bmross.net or by phone at 1-888-524-2641.

Yours very truly

B. M. ROSS AND ASSOCIATES LIMITED

Per

Lisa Courney, RPP, MCIP Environmental Planner

LJC:ss Encl.

cc. Adam Weishar, Municipality of Kincardine

MUNICIPALITY OF KINCARDINE WATER AND WASTEWATER SERVICING MASTER PLAN UPDATE JOB. NO. 22128

REVIEW AGENCY CIRCULATION LIST

Review Agency	Contact Method	Involvement		
Ministry of the Environment, Conservation	Email: agency letter, Notice and Project Information Form (per streamlined process) Mark Badali, Environmental Resource			
and Parks (MECP) – EA Coordinator	Planner & EA Coordinator – Southwest Region Project Review Unit, Environmental Assessment Branch Email: mbadali1@ontario.ca	Mandatory Contact		
Ministry of Natural Resources and Forestry - Midhurst	Mail: agency letter Address: 2284 Nursery Rd, Midhurst ON, L9X 1N8	Potential Impact on Natural Features		
	Email: agency letter, copy of Notice			
Ministry of Tourism, Culture and Sport	401 Bay Street, Suite 1700 Toronto, ON M7A 0A7 Email: karla.barboza@ontario.ca	Potential Impact to Cultural Heritage Features and Archaeological Resources		
Bruce County • Transportation and Environmental Services • Planning and Development Department	Email: agency letter, copy of Notice Address: 30 Park St. Walkerton ON N0G 2V0 - Planning & Development Department E-Mail: bcplpe@brucecounty.on.ca Public Works E-Mail: Amanda Froese, Director of Transportation and Environmental Services afroese@brucecounty.on.ca	 General information Implications for long-term development 		

Review Agency	Contact Method	Involvement
Saugeen Valley Conservation Authority	Email: agency letter, copy of Notice Address: 1078 Bruce Road 12, Box 150 Formosa ON N0G 1W0 Cassandra Malo: c.malo@svca.on.ca	Potential Impact on Natural Features
Municipality of Kincardine	Email: agency letter, copy of Notice 1475 Concession 5, RR 5, Kincardine ON, N2Z 2X6 Adam Weishar, Director of Infrastructure and Development aweishar@kincardine.ca	Proponent
Saugeen, Grey Sauble, Northern Bruce Peninsula Source Water Protection	Email: agency letter, copy of Notice Carl Seider, Project Manager c.seider@waterprotection.ca	Potential Impact related to Source Water Protection
Kincardine Fire Chief	Email: agency letter, copy of Notice Address: Brad Lemaich, Fire Chief 127 Mahood-Johnston Drive Kincardine N2Z 3A2 Brad Lemaich, Fire Chief blemaich@kincardine.ca	Potential impact to service area
Bruce-Grey Public Health Unit	Mail: agency letter, copy of Notice Address: 101 17 th Street East, Owen Sound ON N4K 0A5	Water quality concerns



B. M. ROSS AND ASSOCIATES LIMITED
Engineers and Planners
62 North Street, Goderich, ON N7A 2T4
p. (519) 524-2641 www.bmross.net

File No. 22128

October 28, 2022

Aboriginal community (see attached list)

RE: Municipality of Kincardine Water and Wastewater Servicing Master Plan Update

The Municipality of Kincardine is updating the Water and Wastewater Servicing Master Plan for the communities of Kincardine, Tiverton and Lakeshore area from Kincardine to Inverhuron. The Master Plan was adopted in 2018 and since that time there has been residential and non-residential growth, additional development proposals and new and upgraded infrastructure within the systems. The updated Master Plan will involve a review of existing water treatment, storage, and supply infrastructure, and sanitary sewage collection, pumping, and treatment infrastructure. This update is being completed concurrently with a Municipal Class Environmental Assessment (MCEA) for an expansion to the Kincardine Water Treatment Plant, which is already an anticipated need based on potential community growth in combination with the potential to add Bruce Power as a water customer. Information from the Master Plan will inform and be used in the MCEA.

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Your community was identified as possibly having an interest in this project and we are soliciting your input. Please forward your response to the undersigned by December 15, 2022.

If you have any questions or require further information, please contact the undersigned at lcourtney@bmross.net or by phone at 1-888-524-2641.

Yours very truly

B. M. ROSS AND ASSOCIATES LIMITED

Per ____

isa Courtney, RPP, MCII

Environmental Planner

LJC:hv Encl.

cc. Adam Weishar, Municipality of Kincardine

MUNICIPALITY OF KINCARDINE



WATER AND WASTEWATER SERVICING MASTER PLAN UPDATE

NOTICE OF COMMENCEMENT AND OPEN HOUSE

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Adam Weishar, Director of Infrastructure and Development, Municipality of Kincardine This Notice issued November 2, 2022

MUNICIPALITY OF KINCARDINE WATER AND WASTEWATER MASTER PLAN UPDATE BMROSS FILE 22128

ABORIGINAL COMMUNITIES CIRCULATION LIST – OCTOBER 2022

Aboriginal Community	Contact Method
Métis Nation of Ontario	Email: consultations@metisnation.org DavidD@metisnation.org
Chippewas of Saugeen	Hard Copy: Chief Conrad Ritchie Chippewas of Saugeen 6493 Highway 21, RR 1 Southampton, ON NOH 2L0 Email: critchie@saugeenfirstnation.ca
Chippewas of Nawash	Hard Copy: Chief Veronica Smith Chippewas of Nawash Unceded First Nation R.R. #5 Wiarton, ON N0H 2T0 Email: chief.veronica@nawash.ca
Great Lakes Metis Council	Hard Copy: Great Lakes Metis Council 380 9th Street East Owen Sound, ON N4K 1P1 Email: peterc1908@hotmail.com
Historic Saugeen Métis	Email: hsmlrcc@bmts.com
Saugeen Ojibway Nation Environment Office Juanita Meekins (Executive Assistant) and Emily Martin (Infrastructure and Resource Manager)	Email: manager.ri@saugeenojibwaynation.ca, cc: execassist.ri@saugeenojibwaynation.ca
Chippewas of Kettle and Stony Point First Nation	Hard copy: Chief Jason Henry Chippewas of Kettle and Stony Point First Nation 6247 Indian Lane, RR#2 Forest ON NON 1J1 Email: Jason.henry@kettlepoint.org



Ministry of the Environment, Conservation and Parks

Ministère de l'Environnement, de la Protection de la nature

et des Parcs

Environmental Assessment

Branch

Direction des évaluations environnementales

1st Floor Rez-de-chaussée

 135 St. Clair Avenue W
 135, avenue St. Clair Ouest

 Toronto ON M4V 1P5
 Toronto ON M4V 1P5

 Tel.: 416 314-8001
 Tél.: 416 314-8001

 Fax.: 416 314-8452
 Téléc.: 416 314-8452

November 25, 2022

Adam Weishar
Director of Infrastructure and Development
Municipality of Kincardine
aweishar@kincardine.ca

BY EMAIL ONLY

Re: Water and Wastewater Servicing Master Plan Update

Municipality of Kincardine

Municipal Class Environmental Assessment, Master Plan (Approach #1)

Acknowledgement of Notice of Commencement

Dear Adam Weishar,

This letter is in response to the Notice of Commencement for the above noted Master Plan. The Ministry of the Environment, Conservation and Parks (MECP) acknowledges that the Municipality of Kincardine (proponent) has indicated that the study is following the approved environmental planning process for a Master Plan incorporating Phases 1 and 2 under the Municipal Class Environmental Assessment (Class EA).

The updated (August 2022) attached "Areas of Interest" document provides guidance regarding the ministry's interests with respect to the Class EA process. Please address all areas of interest in the EA documentation at an appropriate level for the EA study. Proponents who address all the applicable areas of interest can minimize potential delays to the project schedule. Further information is provided at the end of the Areas of Interest document relating to recent changes to the Environmental Assessment Act through Bill 197, Covid-19 Economic Recovery Act 2020.

The Crown has a legal duty to consult Aboriginal communities when it has knowledge, real or constructive, of the existence or potential existence of an Aboriginal or treaty right and contemplates conduct that may adversely impact that right. Before authorizing the project(s) identified in this Master Plan, the Crown must ensure that its duty to consult has been fulfilled, where such a duty is triggered. Although the duty to consult with Aboriginal peoples is a duty of the Crown, the Crown may delegate procedural aspects of this duty to project proponents while retaining oversight of the consultation process.

The proposed project(s) may have the potential to affect Aboriginal or treaty rights protected under Section 35 of Canada's *Constitution Act* 1982. Where the Crown's duty to consult is triggered in relation to the proposed project(s), the MECP is delegating the procedural aspects of rights-based consultation to the proponent through this letter. The Crown intends to rely on the delegated consultation process in discharging its duty to consult and maintains the right to participate in the consultation process as it sees fit.

Based on information provided to date and the Crown's preliminary assessment the proponent is required to consult with the following communities who have been identified as potentially affected by the proposed project(s):

- Saugeen First Nation and the Chippewas of Nawash Unceded First Nation
 - These communities work together on consultation issues and are known collectively as the Saugeen Ojibway Nation. They have requested notices be sent to the Saugeen Ojibway Nation Environment Office with a copy to the Chief and Council of Saugeen First Nation and Chippewas of Nawash Unceded First Nation.
- Métis Nation of Ontario- Lands and Resources Dept, Region 7

Steps that the proponent may need to take in relation to Aboriginal consultation for the proposed project are outlined in the "Code of Practice for Consultation in Ontario's Environmental Assessment Process". Additional information related to Ontario's Environmental Assessment Act is available online at: www.ontario.ca/environmentalassessments.

Please also refer to the attached document "A Proponent's Introduction to the Delegation of Procedural Aspects of consultation with Aboriginal Communities" for further information, including the MECP's expectations for EA report documentation related to consultation with communities.

The proponent must contact the Director of Environmental Assessment Branch (EABDirector@ontario.ca) under the following circumstances after initial discussions with the communities identified by the MECP:

- Aboriginal or treaty rights impacts are identified to you by the communities;
- You have reason to believe that your proposed project(s) may adversely affect an Aboriginal or treaty right;

- Consultation with Indigenous communities or other stakeholders has reached an impasse; or
- A Section 16 Order request is expected based on impacts to Aboriginal or treaty rights

The MECP will then assess the extent of any Crown duty to consult for the circumstances and will consider whether additional steps should be taken, including what role you will be asked to play should additional steps and activities be required.

A draft copy of the report should be sent directly to me prior to the filing of the final report, allowing a minimum of 30 days for the ministry's technical reviewers to provide comments.

Please also ensure a copy of the final notice is sent to the ministry's Southwest Region EA notification email account (eanotification.swregion@ontario.ca) after the draft report is reviewed and finalized.

Should you or any members of your project team have any questions regarding the material above, please contact me at mark.badali1@ontario.ca.

Sincerely,

Mark Badali

Mark Bedali

Regional Environmental Planner – Southwest Region

Cc: John Ritchie, Manager, Owen Sound District Office, MECP

Lisa Courtney, Environmental Planner, B. M. Ross and Associates Limited

Enclosed: Areas of Interest

Attached: Client's Guide to Preliminary Screening for Species at Risk

A Proponent's Introduction to the Delegation of Procedural Aspects of Consultation

with Aboriginal Communities

AREAS OF INTEREST (v. August 2022)

It is suggested that you check off each section after you have considered / addressed it.

□ Planning and Policy

- Applicable plans and policies should be identified in the report, and the proponent should describe how the proposed project adheres to the relevant policies in these plans.
 - Projects located in MECP Central, Eastern or West Central Region may be subject to A Place to Grow: Growth Plan for the Greater Golden Horseshoe (2020).
 - Projects located in MECP Central or Eastern Region may be subject to the <u>Oak</u>
 <u>Ridges Moraine Conservation Plan</u> (2017) or the <u>Lake Simcoe Protection Plan</u>
 (2014).
 - Projects located in MECP Central, Southwest or West Central Region may be subject to the <u>Niagara Escarpment Plan</u> (2017).
 - Projects located in MECP Central, Eastern, Southwest or West Central Region may be subject to the <u>Greenbelt Plan</u> (2017).
 - Projects located in MECP Northern Region may be subject to the <u>Growth Plan</u> <u>for Northern Ontario</u> (2011).
- The <u>Provincial Policy Statement</u> (2020) contains policies that protect Ontario's natural heritage and water resources. Applicable policies should be referenced in the report, and the proponent should <u>describe</u> how the proposed project is consistent with these policies.
- In addition to the provincial planning and policy level, the report should also discuss the planning context at the municipal and federal levels, as appropriate.

☐ Source Water Protection

The Clean Water Act, 2006 (CWA) aims to protect existing and future sources of drinking water. To achieve this, several types of vulnerable areas have been delineated around surface water intakes and wellheads for every municipal residential drinking water system that is located in a source protection area. These vulnerable areas are known as a Wellhead Protection Areas (WHPAs) and surface water Intake Protection Zones (IPZs). Other vulnerable areas that have been delineated under the CWA include Highly Vulnerable Aquifers (HVAs), Significant Groundwater Recharge Areas (SGRAs), Event-based modelling areas (EBAs), and Issues Contributing Areas (ICAs). Source protection plans have been developed that include policies to address existing and future risks to sources of municipal drinking water within these vulnerable areas.

Projects that are subject to the Environmental Assessment Act that fall under a Class EA, or one of the Regulations, have the potential to impact sources of drinking water if they occur in designated vulnerable areas or in the vicinity of other at-risk drinking water systems (i.e.

systems that are not municipal residential systems). MEA Class EA projects may include activities that, if located in a vulnerable area, could be a threat to sources of drinking water (i.e. have the potential to adversely affect the quality or quantity of drinking water sources) and the activity could therefore be subject to policies in a source protection plan. Where an activity poses a risk to drinking water, policies in the local source protection plan may impact how or where that activity is undertaken. Policies may prohibit certain activities, or they may require risk management measures for these activities. Municipal Official Plans, planning decisions, Class EA projects (where the project includes an activity that is a threat to drinking water) and prescribed instruments must conform with policies that address significant risks to drinking water and must have regard for policies that address moderate or low risks.

- In October 2015, the MEA Parent Class EA document was amended to include reference to the Clean Water Act (Section A.2.10.6) and indicates that proponents undertaking a Municipal Class EA project must identify early in their process whether a project is or could potentially be occurring with a vulnerable area. **Given this requirement, please include a section in the report on source water protection.**
 - The proponent should identify the source protection area and should clearly document how the proximity of the project to sources of drinking water (municipal or other) and any delineated vulnerable areas was considered and assessed.
 Specifically, the report should discuss whether or not the project is located in a vulnerable area and provide applicable details about the area.
 - o If located in a vulnerable area, proponents should document whether any project activities are prescribed drinking water threats and thus pose a risk to drinking water (this should be consulted on with the appropriate Source Protection Authority). Where an activity poses a risk to drinking water, the proponent must document and discuss in the report how the project adheres to or has regard to applicable policies in the local source protection plan. This section should then be used to inform and be reflected in other sections of the report, such as the identification of net positive/negative effects of alternatives, mitigation measures, evaluation of alternatives etc.
- While most source protection plans focused on including policies for significant drinking
 water threats in the WHPAs and IPZs it should be noted that even though source protection
 plan policies may not apply in HVAs, these are areas where aquifers are sensitive and at risk
 to impacts and within these areas, activities may impact the quality of sources of drinking
 water for systems other than municipal residential systems.
- In order to determine if this project is occurring within a vulnerable area, proponents can use Source Protection Information Atlas, which is an online mapping tool available to the public. Note that various layers (including WHPAs, WHPA-Q1 and WHPA-Q2, IPZs, HVAs, SGRAs, EBAs, ICAs) can be turned on through the "Map Legend" bar on the left. The

mapping tool will also provide a link to the appropriate source protection plan in order to identify what policies may be applicable in the vulnerable area.

 For further information on the maps or source protection plan policies which may relate to their project, proponents must contact the appropriate source protection authority. Please consult with the local source protection authority to discuss potential impacts on drinking water. Please document the results of that consultation within the report and include all communication documents/correspondence.

More Information

For more information on the *Clean Water Act*, source protection areas and plans, including specific information on the vulnerable areas and drinking water threats, please refer to Conservation Ontario's website where you will also find links to the local source protection plan/assessment report.

A list of the prescribed drinking water threats can be found in <u>section 1.1 of Ontario Regulation 287/07</u> made under the *Clean Water Act*. In addition to prescribed drinking water threats, some source protection plans may include policies to address additional "local" threat activities, as approved by the MECP.

☐ Climate Change

The document "Considering Climate Change in the Environmental Assessment Process" (Guide) is now a part of the Environmental Assessment program's Guides and Codes of Practice. The Guide sets out the MECP's expectation for considering climate change in the preparation, execution and documentation of environmental assessment studies and processes. The guide provides examples, approaches, resources, and references to assist proponents with consideration of climate change in EA. Proponents should review this Guide in detail.

The MECP expects proponents of Class EA projects to:

- Consider during the assessment of alternative solutions and alternative designs, the following:
 - a. the project's expected production of greenhouse gas emissions and impacts on carbon sinks (climate change mitigation); and
 - b. resilience or vulnerability of the undertaking to changing climatic conditions (climate change adaptation).
- 2. Include a discrete section in the report detailing how climate change was considered in the EA.

How climate change is considered can be qualitative or quantitative in nature and should be scaled to the project's level of environmental effect. In all instances, both a project's impacts on climate change (mitigation) and impacts of climate change on a project (adaptation) should be considered.

• The MECP has also prepared another guide to support provincial land use planning direction related to the completion of energy and emission plans. The "Community Emissions Reduction Planning: A Guide for Municipalities" document is designed to educate stakeholders on the municipal opportunities to reduce energy and greenhouse gas emissions, and to provide guidance on methods and techniques to incorporate consideration of energy and greenhouse gas emissions into municipal activities of all types. We encourage you to review the Guide for information.

☐ Air Quality, Dust and Noise

- If there are sensitive receptors in the surrounding area of this project, a quantitative air quality/odour impact assessment will be useful to evaluate alternatives, determine impacts and identify appropriate mitigation measures. The scope of the assessment can be determined based on the potential effects of the proposed alternatives, and typically includes source and receptor characterization and a quantification of local air quality impacts on the sensitive receptors and the environment in the study area. The assessment will compare to all applicable standards or guidelines for all contaminants of concern.
 Please contact this office for further consultation on the level of Air Quality Impact Assessment required for this project if not already advised.
- If a quantitative Air Quality Impact Assessment is not required for the project, the MECP expects that the report contain a qualitative assessment which includes:
 - A discussion of local air quality including existing activities/sources that significantly impact local air quality and how the project may impact existing conditions;
 - A discussion of the nearby sensitive receptors and the project's potential air quality impacts on present and future sensitive receptors;
 - A discussion of local air quality impacts that could arise from this project during both construction and operation; and
 - A discussion of potential mitigation measures.
- As a common practice, "air quality" should be used an evaluation criterion for all road projects.
- Dust and noise control measures should be addressed and included in the construction
 plans to ensure that nearby residential and other sensitive land uses within the study area
 are not adversely affected during construction activities.
- The MECP recommends that non-chloride dust-suppressants be applied. For a comprehensive list of fugitive dust prevention and control measures that could be applied, refer to <u>Cheminfo Services Inc. Best Practices for the Reduction of Air Emissions from</u>

<u>Construction and Demolition Activities</u> report prepared for Environment Canada. March 2005.

• The report should consider the potential impacts of increased noise levels during the operation of the completed project. The proponent should explore all potential measures to mitigate significant noise impacts during the assessment of alternatives.

☐ Ecosystem Protection and Restoration

- Any impacts to ecosystem form and function must be avoided where possible. The report should describe any proposed mitigation measures and how project planning will protect and enhance the local ecosystem.
- Natural heritage and hydrologic features should be identified and described in detail to assess potential impacts and to develop appropriate mitigation measures. The following sensitive environmental features may be located within or adjacent to the study area:
 - Key Natural Heritage Features: Habitat of endangered species and threatened species, fish habitat, wetlands, areas of natural and scientific interest (ANSIs), significant valleylands, significant woodlands; significant wildlife habitat (including habitat of special concern species); sand barrens, savannahs, and tallgrass prairies; and alvars.
 - Key Hydrologic Features: Permanent streams, intermittent streams, inland lakes and their littoral zones, seepage areas and springs, and wetlands.
 - Other natural heritage features and areas such as: vegetation communities, rare species of flora or fauna, Environmentally Sensitive Areas, Environmentally Sensitive Policy Areas, federal and provincial parks and conservation reserves, Greenland systems etc.

We recommend consulting with the Ministry of Natural Resources and Forestry (MNRF), Fisheries and Oceans Canada (DFO) and your local conservation authority to determine if special measures or additional studies will be necessary to preserve and protect these sensitive features. In addition, for projects located in Central Region you may consider the provisions of the Rouge Park Management Plan if applicable.

☐ Species at Risk

- The Ministry of the Environment, Conservation and Parks has now assumed responsibility of Ontario's Species at Risk program. Information, standards, guidelines, reference materials and technical resources to assist you are found at https://www.ontario.ca/page/speciesrisk.
- The Client's Guide to Preliminary Screening for Species at Risk (Draft May 2019) has been attached to the covering email for your reference and use. Please review this document for next steps.

• For any questions related to subsequent permit requirements, please contact SAROntario@ontario.ca.

☐ Surface Water

- The report must include enough information to demonstrate that there will be no negative
 impacts on the natural features or ecological functions of any watercourses within the study
 area. Measures should be included in the planning and design process to ensure that any
 impacts to watercourses from construction or operational activities (e.g. spills, erosion,
 pollution) are mitigated as part of the proposed undertaking.
- Additional stormwater runoff from new pavement can impact receiving watercourses and flood conditions. Quality and quantity control measures to treat stormwater runoff should be considered for all new impervious areas and, where possible, existing surfaces. The ministry's Stormwater Management Planning and Design Manual (2003) should be referenced in the report and utilized when designing stormwater control methods. A Stormwater Management Plan should be prepared as part of the Class EA process that includes:
 - Strategies to address potential water quantity and erosion impacts related to stormwater draining into streams or other sensitive environmental features, and to ensure that adequate (enhanced) water quality is maintained
 - Watershed information, drainage conditions, and other relevant background information
 - Future drainage conditions, stormwater management options, information on erosion and sediment control during construction, and other details of the proposed works
 - Information on maintenance and monitoring commitments.
- Ontario Regulation 60/08 under the Ontario Water Resources Act (OWRA) applies to the
 Lake Simcoe Basin, which encompasses Lake Simcoe and the lands from which surface
 water drains into Lake Simcoe. If the proposed sewage treatment plant is listed in Table 1 of
 the regulation, the report should describe how the proposed project and its mitigation
 measures are consistent with the requirements of this regulation and the OWRA.
- Any potential approval requirements for surface water taking or discharge should be identified in the report. A Permit to Take Water (PTTW) under the OWRA will be required for any water takings that exceed 50,000 L/day, except for certain water taking activities that have been prescribed by the Water Taking EASR Regulation – O. Reg. 63/16. These prescribed water-taking activities require registration in the EASR instead of a PTTW. Please

review the <u>Water Taking User Guide for EASR</u> for more information. Additionally, an Environmental Compliance Approval under the OWRA is required for municipal stormwater management works.

☐ Groundwater

- The status of, and potential impacts to any well water supplies should be addressed. If the
 project involves groundwater takings or changes to drainage patterns, the quantity and
 quality of groundwater may be affected due to drawdown effects or the redirection of
 existing contamination flows. In addition, project activities may infringe on existing wells
 such that they must be reconstructed or sealed and abandoned. Appropriate information to
 define existing groundwater conditions should be included in the report.
- If the potential construction or decommissioning of water wells is identified as an issue, the report should refer to Ontario Regulation 903, Wells, under the OWRA.
- Potential impacts to groundwater-dependent natural features should be addressed. Any
 changes to groundwater flow or quality from groundwater taking may interfere with the
 ecological processes of streams, wetlands or other surficial features. In addition,
 discharging contaminated or high volumes of groundwater to these features may have
 direct impacts on their function. Any potential effects should be identified, and appropriate
 mitigation measures should be recommended. The level of detail required will be
 dependent on the significance of the potential impacts.
- Any potential approval requirements for groundwater taking or discharge should be identified in the report. A Permit to Take Water (PTTW) under the OWRA will be required for any water takings that exceed 50,000 L/day, with the exception of certain water taking activities that have been prescribed by the Water Taking EASR Regulation O. Reg. 63/16. These prescribed water-taking activities require registration in the EASR instead of a PTTW. Please review the Water Taking User Guide for EASR for more information.
- Consultation with the railroad authorities is necessary wherever there is a plan to use construction dewatering in the vicinity of railroad lines or where the zone of influence of the construction dewatering potentially intercepts railroad lines.

■ Excess Materials Management

• In December 2019, MECP released a new regulation under the Environmental Protection Act, titled "On-Site and Excess Soil Management" (O. Reg. 406/19) to support improved management of excess construction soil. This regulation is a key step to support proper management of excess soils, ensuring valuable resources don't go to waste and to provide

clear rules on managing and reusing excess soil. New risk-based standards referenced by this regulation help to facilitate local beneficial reuse which in turn will reduce greenhouse gas emissions from soil transportation, while ensuring strong protection of human health and the environment. The new regulation is being phased in over time, with the first phase in effect on January 1, 2021. For more information, please visit https://www.ontario.ca/page/handling-excess-soil.

- The report should reference that activities involving the management of excess soil should be completed in accordance with O. Reg. 406/19 and the MECP's current guidance document titled "Management of Excess Soil – A Guide for Best Management Practices" (2014).
- All waste generated during construction must be disposed of in accordance with ministry requirements

□ Contaminated Sites

- Any current or historical waste disposal sites should be identified in the report. The status of
 these sites should be determined to confirm whether approval pursuant to Section 46 of
 the EPA may be required for land uses on former disposal sites. We recommend referring to
 the MECP's D-4 guideline for land use considerations near landfills and dumps.
 - Resources available may include regional/local municipal official plans and data; provincial data on <u>large landfill sites</u> and <u>small landfill sites</u>; Environmental Compliance Approval information for waste disposal sites on <u>Access Environment</u>.
- Other known contaminated sites (local, provincial, federal) in the study area should also be identified in the report (Note – information on federal contaminated sites is found on the Government of Canada's website).
- The location of any underground storage tanks should be investigated in the report.
 Measures should be identified to ensure the integrity of these tanks and to ensure an appropriate response in the event of a spill. The ministry's Spills Action Centre must be contacted in such an event.
- Since the removal or movement of soils may be required, appropriate tests to determine contaminant levels from previous land uses or dumping should be undertaken. If the soils are contaminated, you must determine how and where they are to be disposed of, consistent with Part XV.1 of the Environmental Protection Act (EPA) and Ontario Regulation 153/04, Records of Site Condition, which details the new requirements related to site assessment and clean up. Please contact the appropriate MECP District Office for further consultation if contaminated sites are present.

□ Servicing, Utilities and Facilities

- The report should identify any above or underground utilities in the study area such as transmission lines, telephone/internet, oil/gas etc. The owners should be consulted to discuss impacts to this infrastructure, including potential spills.
- The report should identify any servicing infrastructure in the study area such as wastewater, water, stormwater that may potentially be impacted by the project.
- Any facility that releases emissions to the atmosphere, discharges contaminants to ground
 or surface water, provides potable water supplies, or stores, transports or disposes of waste
 must have an Environmental Compliance Approval (ECA) before it can operate lawfully.
 Please consult with MECP's Environmental Permissions Branch to determine whether a new
 or amended ECA will be required for any proposed infrastructure.
- We recommend referring to the ministry's <u>environmental land use planning guides</u> to
 ensure that any potential land use conflicts are considered when planning for any
 infrastructure or facilities related to wastewater, pipelines, landfills or industrial uses.

☐ Mitigation and Monitoring

- Contractors must be made aware of all environmental considerations so that all
 environmental standards and commitments for both construction and operation are met.
 Mitigation measures should be clearly referenced in the report and regularly monitored
 during the construction stage of the project. In addition, we encourage proponents to
 conduct post-construction monitoring to ensure all mitigation measures have been effective
 and are functioning properly.
- Design and construction reports and plans should be based on a best management approach that centres on the prevention of impacts, protection of the existing environment, and opportunities for rehabilitation and enhancement of any impacted areas.
- The proponent's construction and post-construction monitoring plans must be documented in the report, as outlined in Section A.2.5 and A.4.1 of the MEA Class EA parent document.

☐ Consultation

• The report must demonstrate how the consultation provisions of the Class EA have been fulfilled, including documentation of all stakeholder consultation efforts undertaken during the planning process. This includes a discussion in the report that identifies concerns that were raised and describes how they have been addressed by the proponent throughout

the planning process. The report should also include copies of comments submitted on the project by interested stakeholders, and the proponent's responses to these comments (as directed by the Class EA to include full documentation).

• Please include the full stakeholder distribution/consultation list in the documentation.

☐ Class EA Process

- If this project is a Master Plan: there are several different approaches that can be used to conduct a Master Plan, examples of which are outlined in Appendix 4 of the Class EA. The Master Plan should clearly indicate the selected approach for conducting the plan, by identifying whether the levels of assessment, consultation and documentation are sufficient to fulfill the requirements for Schedule B or C projects. Please note that any Schedule B or C projects identified in the plan would be subject to Part II Order Requests under the Environmental Assessment Act, although the plan itself would not be. Please include a description of the approach being undertaken (use Appendix 4 as a reference).
- If this project is a Master Plan: Any identified projects should also include information on the MCEA schedule associated with the project.
- The report should provide clear and complete documentation of the planning process in order to allow for transparency in decision-making.
- The Class EA requires the consideration of the effects of each alternative on all aspects of
 the environment (including planning, natural, social, cultural, economic, technical). The
 report should include a level of detail (e.g. hydrogeological investigations, terrestrial and
 aquatic assessments, cultural heritage assessments) such that all potential impacts can be
 identified, and appropriate mitigation measures can be developed. Any supporting studies
 conducted during the Class EA process should be referenced and included as part of the
 report.
- Please include in the report a list of all subsequent permits or approvals that may be required for the implementation of the preferred alternative, including but not limited to, MECP's PTTW, EASR Registrations and ECAs, conservation authority permits, species at risk permits, MTO permits and approvals under the *Impact Assessment Act*, 2019.
- Ministry guidelines and other information related to the issues above are available at http://www.ontario.ca/environment-and-energy/environment-and-energy. We encourage you to review all the available guides and to reference any relevant information in the report.

Amendments to the EAA through the Covid-19 Economic Recovery Act, 2020

Once the EA Report is finalized, the proponent must issue a Notice of Completion providing a minimum 30-day period during which documentation may be reviewed and comment and input can be submitted to the proponent. The Notice of Completion must be sent to the appropriate MECP Regional Office email address.

The public can request a higher level of assessment on a project if they are concerned about potential adverse impacts to constitutionally protected Aboriginal and treaty rights. In addition, the Minister may issue an order on his or her own initiative within a specified time period. The Director (of the Environmental Assessment Branch) will issue a Notice of Proposed Order to the proponent if the Minister is considering an order for the project within 30 days after the conclusion of the comment period on the Notice of Completion. At this time, the Director may request additional information from the proponent. Once the requested information has been received, the Minister will have 30 days within which to make a decision or impose conditions on your project.

Therefore, the proponent cannot proceed with the project until at least 30 days after the end of the comment period provided for in the Notice of Completion. Further, the proponent may not proceed after this time if:

- a Section 16 Order request has been submitted to the ministry regarding potential adverse impacts to constitutionally protected Aboriginal and treaty rights, or
- the Director has issued a Notice of Proposed order regarding the project.

Please ensure that the Notice of Completion advises that outstanding concerns are to be directed to the proponent for a response, and that in the event there are outstanding concerns regarding potential adverse impacts to constitutionally protected Aboriginal and treaty rights, Section 16 Order requests on those matters should be addressed in writing to:

Minister David Piccini
Ministry of Environment, Conservation and Parks
777 Bay Street, 5th Floor
Toronto ON M7A 2J3
minister.mecp@ontario.ca

and

Director, Environmental Assessment Branch Ministry of Environment, Conservation and Parks 135 St. Clair Ave. W, 1st Floor Toronto ON, M4V 1P5 EABDirector@ontario.ca



SENT ELECTRONICALLY ONLY: lcourtney@bmross.net

November 23, 2022

B. M. ROSS AND ASSOCIATES LIMITEDEngineers and Planners62 North Street, Goderich, ON N7A 2T4

ATTENTION: Lisa Courtney, Environmental Planner

Dear Ms. Courtney,

RE: Municipality of Kincardine

Water and Wastewater Servicing Master Plan Update

Further to your letter of October 28, 2022 regarding the above referenced matter, the Saugeen Valley Conservation Authority (SVCA) staff has reviewed the notice of commencement. SVCA were involved with the 2018 version of the Water and Wastewater Servicing Master Plan, and appreciate that the SVCA was included at this time.

As the matter progresses, SVCA staff will identify areas in the Master Plan where SVCA input will be required such as where the works may require SVCA permit(s) pursuant to our Ontario Regulation 169/06, as amended.

Again, SVCA staff thank you for the opportunity to provide our comment and will appreciate the opportunities to review the details of the matter as it continues. Accordingly, we request that you continue to notify the SVCA as subsequent steps arrive. If you have any questions, do not hesitate to contact our office.

Sincerely,

Michael Oberle

Environmental Planning Coordinator

Saugeen Conservation

Michael Obele

MO\

cc: Adam Weishar, Director of Infrastructure, Municipality of Kincardine (via email)

Maureen Couture, SVCA Member representing the Municipality of Kincardine (via email)

Bill Stewart, SVCA Member representing the Municipality of Kincardine (via email)



Ministry of Citizenship and Multiculturalism

Ministère des Affaires civiques et du Multiculturalisme

Heritage Branch 400 University Ave. 5th Floor

Toronto ON M7A 2E7 Tel.: 613.242.3743 Direction du patrimoine 400, av. University 5th étage Toronto ON M7A 2E7

Tél.: 613.242.3743



November 14, 2022

EMAIL ONLY

Lisa J. Courtney, MSc., MCIP, RPP B. M. Ross and Associates Limited Engineers and Planners 62 North Street Goderich, ON N7A 2T4 Icourtney@bmross.net

MCM File : 0018004

Proponent : Municipality of Kincardine

Subject : Notice of Commencement - Master Plan Approach 1
Project : Water and Wastewater Servicing Master Plan Update

Location: Municipality of Kincardine

Dear Lisa J. Courtney:

Thank you for providing us with the Notice of Commencement for this project.

Please note that the responsibility for administration of the *Ontario Heritage Act* and matters related to cultural heritage recently transferred from the Ministry of Tourism, Culture and Sport (MTCS) to the Ministry of Citizenship and Multiculturalism (MCM). Individual staff roles and contact information remain unchanged. Please continue to send any notices, report and/or documentation to both Karla Barboza and myself.

MCM's interest in this master plan relates to it's mandate of conserving Ontario's cultural heritage, which includes archaeological resources, built heritage resources and cultural heritage landscapes.

MCM understands that master plans are long range plans which integrate infrastructure requirements for existing and future land use with environmental assessment planning principles. The Municipal Class Environmental Assessment (MCEA) outlines a framework for master plan and associated studies which should recognize the planning and design Process of this Class EA, and should incorporate the key principles of successful environmental assessment planning identified in Section A.1.1. The master planning process will, at minimum, address Phases 1 and 2 of the Planning and Design Process of the MCEA.

This letter provides advice on how to incorporate consideration of cultural heritage in the abovementioned master planning process by outlining the technical cultural heritage studies and the level of detail required to address cultural heritage in master plans. In accordance with the MCEA, cultural heritage resources should be identified early in the process in order to determine known and potential resources and potential impacts.

Master Plan Summary

The Municipality of Kincardine is updating the Water and Wastewater Servicing Master Plan for the communities of Kincardine, Tiverton and Lakeshore area from Kincardine to Inverhuron. This project is following the process for Master Plans under the MCEA. The proposed approach for this Master Plan (Approach 1) is a broad level assessment that will be the basis for and used in support of future investigations of specific projects.

Identifying Cultural Heritage Resources

MCM understands that the master plan would typically be done at a broad level of assessment thereby requiring more detailed investigations at the project-specific level. Therefore, a description of the existing conditions related to cultural heritage resources needs to be included in the master plan document.

Archaeological Resources

The existing conditions sub-section should indicate if the master plan includes areas of archaeological potential or not and acknowledge that archaeological assessments will be required for future project-specific projects. The proponents should refer to an archaeological management plan or a data sharing agreement, should they exist. In their absence, MTCS's screening checklists can help determine whether archaeological assessments will be needed for subsequent project undertakings: <u>Criteria for Evaluating Archaeological Potential</u> and <u>Criteria for Evaluating Marine Archaeological Potential</u>.

A statement should be included that archaeological assessments are to be undertaken by an archaeologist licensed under the Ontario Heritage Act and that archaeological assessment reports must be submitted for MCM review prior to the completion of the environmental assessment and prior to any ground disturbance. Some municipalities may also elect to have a Stage 1 archaeological assessment undertaken for a master plan area.

Built Heritage Resources and Cultural Heritage Landscapes

MCM recommends that an Existing Conditions Report be undertaken by a qualified person, which will include a historical summary of the study area's development, identifying all known or potential built heritage resources and cultural heritage landscapes within the study area. The findings of the existing conditions report should be included in the existing conditions subsection of the master plan document.

Community input should be sought to identify locally recognized and potential cultural heritage resources. Sources include, but are not limited to, Municipal Heritage Committees, community heritage registers, historical societies and other local heritage organizations.

Cultural heritage resources are often of critical importance to Indigenous communities. Indigenous communities may have knowledge that can contribute to the identification of cultural heritage resources, and any engagement with Indigenous communities should include a discussion about known or potential cultural heritage resources that are of value to them.

Subsequent Municipal Class EA Undertakings

The recommendations outlined above can be used in support of any future technical cultural heritage studies required for any Schedule B and C MCEA undertakings identified within the master planning area. Technical cultural heritage studies are to be undertaken by a qualified person who has expertise, recent experience, and knowledge relevant to the type of cultural heritage resources being considered and the nature of the activity being proposed. Please advise MCM whether any technical cultural heritage studies will be completed for this master plan and provide them to MCM before issuing a Notice of Completion.

Thank you for consulting MCM on this project. Please continue to do so through the master plan process. Do no hesitate to contact me with any questions or clarification.

Sincerely,

Joseph Harvey Heritage Planner joseph.harvey@ontario.ca

It is the sole responsibility of proponents to ensure that any information and documentation submitted as part of their EA report or file is accurate. The Ministry of Citizenship and Multiculturalism (MCM) makes no representation or warranty as to the completeness, accuracy or quality of the any checklists, reports or supporting documentation submitted as part of the EA process, and in no way shall MCM be liable for any harm, damages, costs, expenses, losses, claims or actions that may result if any checklists, reports or supporting documents are discovered to be inaccurate, incomplete, misleading or fraudulent.

Should previously undocumented archaeological resources be discovered, they may be a new archaeological site and therefore subject to Section 48(1) of the *Ontario Heritage Act*. The proponent or person discovering the archaeological resources must cease alteration of the site immediately and engage a licensed consultant archaeologist to carry out an archaeological assessment, in compliance with Section 48(1) of the *Ontario Heritage Act*.

The Funeral, Burial and Cremation Services Act, 2002, S.O. 2002, c.33 requires that any person discovering human remains must cease all activities immediately and notify the police or coroner. If the coroner does not suspect foul play in the disposition of the remains, in accordance with Ontario Regulation 30/11 the coroner shall notify the Registrar, Ontario Ministry of Public and Business Service Delivery, which administers provisions of that Act related to burial sites. In situations where human remains are associated with archaeological resources, the Ministry of Citizenship and Multiculturalism should also be notified (at archaeology@ontario.ca) to ensure that the archaeological site is not subject to unlicensed alterations which would be a contravention of the Ontario Heritage Act.

From: <u>Coreena Smith</u>

To: <u>Klarika Hamer</u>; <u>Lisa Courtney</u>

Cc: <u>Jack Van Dorp; Amanda Froese; Claire Dodds</u>

Subject: RE: 22128 Notice of Study Commencement Municipality of Kincardine Water and Wastewater Master Plan

Date: November 23, 2022 4:11:44 PM

Attachments: <u>image001.jpg</u>

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22128-2022-10-28-BC let.pdf

Lisa,

Thank you for forwarding to the County's attention the Notice of Study Commencement for Kincardine's Water and Wastewater Master Plan Update. We are happy to be engaged in this project and are available to provide support to the Municipality (e.g., attendance at public meetings if required, provision of the County' current growth projection numbers in light of our current inprogress update to the County Official Plan).

Please do not hesitate to reach out.

Kind regards,

Coreena Smith

Senior Planner
Planning and Development
Corporation of the County of Bruce

Office: 519-881-1782 Direct: 226-909-6305

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YuSP8QUKDbHlh2AMyEw?u=http://www.brucecounty.on.ca/



From: Klarika Hamer < KHamer@brucecounty.on.ca>

Sent: Tuesday, November 8, 2022 9:02 AM **To:** Lisa Courtney courtney@bmross.net>

Cc: Coreena Smith <CJSmith@brucecounty.on.ca>

Subject: RE: 22128 Notice of Study Commencement Municipality of Kincardine Water and

Wastewater Master Plan

Good morning Lisa,

Thank you for your email.

We are in receipt of the above noted Notice and will provide input by November 23, 2022.

Coreena Smith is the Planner on this file and is copied on this email.

Kind regards,

Klarika Hamer

Applications Technician
Planning and Development
Corporation of the County of Bruce

Office: 226-909-3359

https://link.edgepilot.com/s/dc78aae5/W0-

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From: Lisa Courtney < lcourtney@bmross.net Sent: Tuesday, November 1, 2022 11:32 AM

To: Bruce County Planning - Lakeshore Hub < bcplpe@brucecounty.on.ca; Amanda Froese AFroese@brucecounty.on.ca;

Subject: 22128 Notice of Study Commencement Municipality of Kincardine Water and Wastewater Master Plan

** [CAUTION]: This email originated from outside of the organization. Do not click links or open attachments unless you recognize the sender and know the content is safe.

Hello,

Please find attached the Notice of Study Commencement for the update to the Municipality of Kincardine Water and Wastewater Master Plan. If you have any initial questions or comments, please do not hesitate to reach out.

Thanks and cheers,

Lisa J. Courtney, MSc., MCIP, RPP B. M. Ross and Associates Limited Engineers and Planners 62 North Street Goderich, ON N7A 2T4

Ph: (519) 524-2641 lcourtney@bmross.net

https://link.edgepilot.com/s/a97a679c/9GxU2F5BZU_BhHL2elfrcg?u=http://www.bmross.net/

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Lisa Courtney

From: Coordinator LRC HSM <hsmlrcc@bmts.com>

Sent: November 9, 2022 9:49 AM

To: Lisa Courtney

Subject: Request for Comments - Kincardine - Water and Wastewater Servicing Master Plan

Update

Kincardine Municipality (projects)

RE: File No. 22128

The Historic Saugeen Métis (HSM) Lands, Resources and Consultation Department is interested to receive further information regarding the Kincardine Water and Wastewater Servicing Master Plan Update as additional information becomes available.

Thank you for the opportunity to review this matter.

Regards,

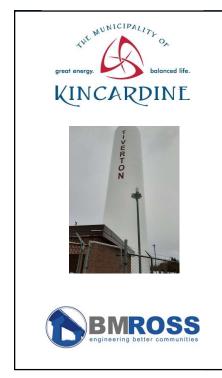
Chris Hachey

Coordinator, Lands, Resources & Consultation

Historic Saugeen Métis email: hsmlrcc@bmts.com phone: 519-483-4000 site: saugeenmetis.com

address: 204 High Street Southampton, ON

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Municipality of Kincardine Water & Wastewater Servicing Master Plan Update

PUBLIC OPEN HOUSE NOVEMBER 29, 2022 6 PM – 8 PM

1

What are Master Plans?

- Master Plans are long range plans that look at integrated infrastructure systems (like water and wastewater systems) over large geographic areas.
- Under the Municipal Class Environmental Assessment (Class EA) process, Master Plans can be customised to suit needs they can be broad in scope and general in details, or can examine strategies or alternatives in detail.
- Strategies identified in Master Plan can be incorporated into future Official Plan and Zoning By law updates.
- Can be a living document update and review on a regular basis.
- Servicing Master Plans look at infrastructure systems to identify and evaluate current and future water and wastewater needs.
 - Identify infrastructure projects, timing, and potential costs.
 - Think of a Master Plan as a roadmap or gameplan for the future, to allow the Municipality to plan and budget for projects.

2018 Master Plan

- Technical analyses completed in 2017 and report issued in spring of 2018.
- Examined proposed developments, growth forecasts and existing infrastructure to identify water and wastewater projects.
 - ▶ Water study areas based on treatment plant servicing area:
 - Kincardine, including Lakeshore (Kincardine north to Inverhuron)
 - ► Tiverton
 - Wastewater study areas based on treatment plant servicing area:
 - Kincardine
 - Bruce Energy Centre (BEC) services Tiverton, Inverhuron, BEC Business Park
- Projects identified as needed now (2018), in 3 5 years and over the longer term (depending on the progression of development).
- 2022 Master Plan will be an update to the previously completed analyses.

3

Recommended Projects – 2018 Master Plan

SYSTEM	PROJECT	TIMING (IN 2018 REPORT)	STATUS
Kincardine Water	WTP Disinfection – UV	Within 5 years	Pending plant expansion
Kincardine Water	Standpipe BPS Rehab	Within 5 years	Done
Kincardine Water	Trunk watermain – Sutton, Russell,	2018	Done
	Kincardine Ave.		
Kincardine Water	Gary Street BPS	2018	Done
Tiverton Water	Review PTTW/MDWL discrepancy	Within 3 years	2020 PTTW renewal; capacity maintained
Tiverton Water	Standpipe BPS Rehab	Within 5 years	Done
Tiverton Water	King St. Watermain	Depend on development	Depend on development
Kincardine Wastewater	Durham St. SPS Design & Approvals	2018	Durham St. 2022 design for 2023 construction, Park St.
Kincardine Wastewater	Park St. SPS Design & Approvals	2018	2023 design for 2024 construction
Kincardine Wastewater	Huron Terrace SPS Design	2018	Done
Kincardine Wastewater	SPS/WWTP SCADA Control	Depend on Municipal	Complete at most sites, and plans in place for remaining
		direction	sites
Kincardine Wastewater	Durham St. sewer	Depend on development	Done
Kincardine Wastewater	Queen St. North and Kingsway sewer	Depend on development	Depend on development
Kincardine Wastewater	Russel St. sewer	Depend on development	Done
Kincardine Wastewater	Gary, Sutton, Mechanics, James	2018	Done
BEC and Tiverton Wastewater	Maple St. SPS Analysis & Upgrade	Within 5 years	Flow study complete; no upgrades required at this time.
BLC and fiverion wastewater	Design Operation	vvicinii 3 years	Thow study complete, no upgrades required at this time.
BEC and Tiverton Wastewater	SPS SCADA Control	Depend on Municipal	Complete at most sites, and plans in place for remaining
		direction	sites

Study Areas

Study Areas are generally based on treatment plant service areas.

Water

- Kincardine and Lakeshore Area (north to Inverhuron)
 - Bruce Power is being considered as a potential customer. A Class EA related to treatment plant expansion is underway.
- Tiverton

Wastewater

- Kincardine
- BEC Wastewater Treatment Plant (WWTP)
 - services Tiverton, Inverhuron, BEC Business Park and Concession 2 Industrial Lands.

5

Growth and Development

- Population and Equivalent Residential Units (ERU) forecasted
 - ▶ 1 single detached house (2.48 persons per unit) 1 ERU
 - ▶ 1 multi unit (2.01 persons per unit) 0.85 ERU
 - ▶ 1 apartment (1.38 persons per unit) 0.60 ERU
- Utilized existing forecasts:
 - ▶ 2021 Kincardine Official Plan
 - ▶ Ministry of Finance (for Bruce County)
 - ▶ 2021 Development Charge Background Study
 - ▶ Bruce County Good Growth Discussion Paper



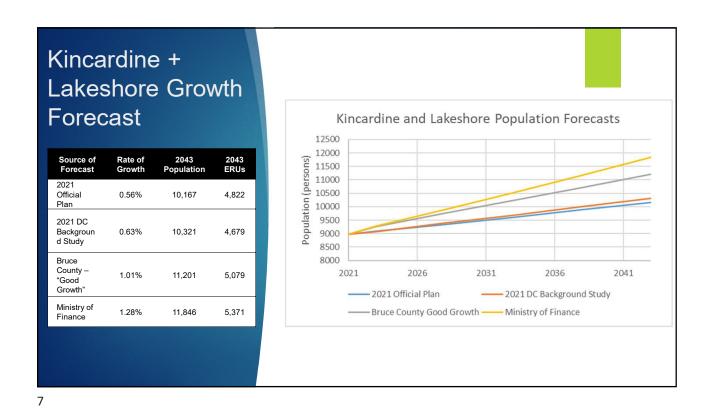






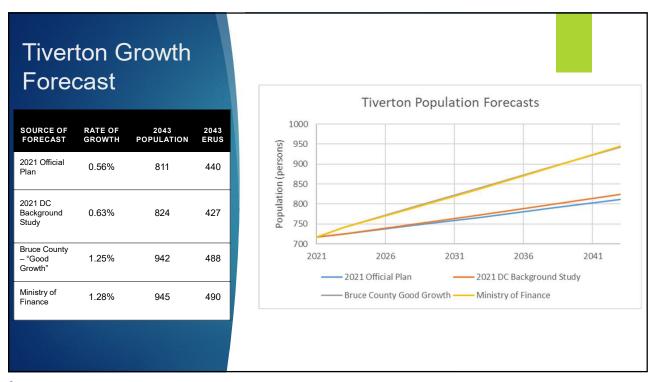


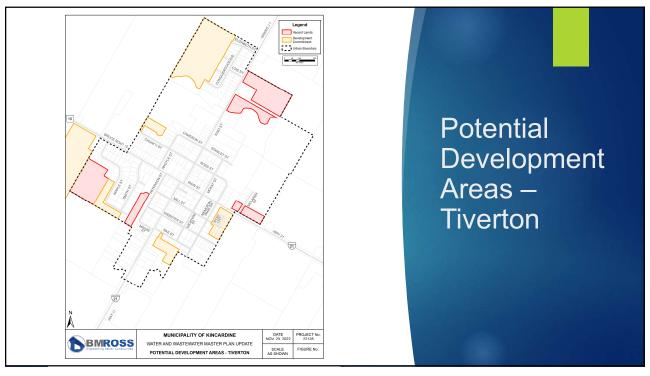


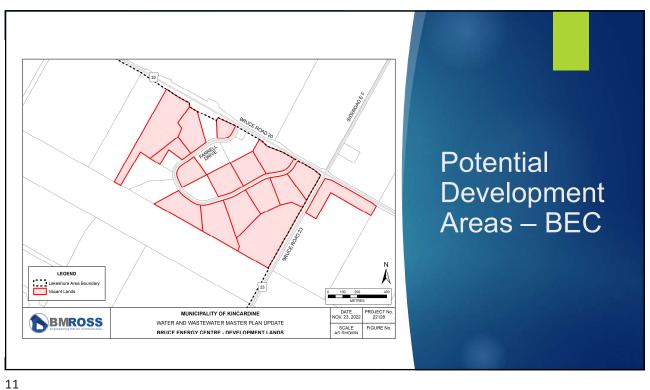


Potential Development Areas - Kincardine

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WINDRAFT COMMITTER PLAN UPDATE
BEYELOPMENT COMMITTER P







Kincardine Water **Treatment**

- ▶ Rated capacity is 11,563 m³/day
- ► Current customers: 4,073 includes Kincardine, Huron Kinloss properties, and Lakeshore north to Inverhuron. Current maximum demand is 6,954 m³/day (or 1.71 m³/day per customer).
- ▶ Approximately 60% of capacity being utilized
- ► Committed capacity 1,624 ERUs x 1.71 m³/day 2,773 m³/day
- ▶ Uncommitted capacity 1,836 m³/day or 1,076 **ERUs**



Water Supply to Bruce Power

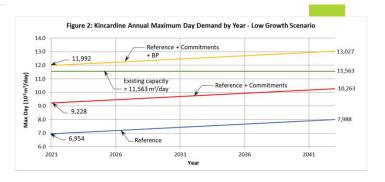
- ▶ Require approximately 2,765 m³/day.
- Uncommitted capacity is inadequate to supply Bruce Power.
 - ▶ Servicing Bruce Power would require additional treatment capacity at the water treatment plant.
- A Municipal Class Environmental Assessment currently underway to evaluate impacts of supplying Bruce Power from the Kincardine Drinking Water System (DWS).

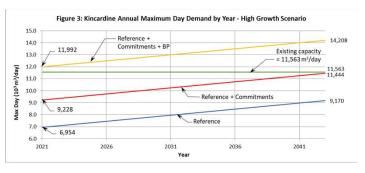


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Kincardine Water Treatment – Forecasted Demand

- Capacity expected to be sufficient to handle growth & development commitments over the next 20 years for both growth scenarios, without Bruce Power as a customer.
- The Municipality could maintain the same number of commitments (i.e., inventory) as it currently has (approximately 1,140 residential units) in either scenario.
- The addition of the Bruce Power would result in committed demand immediately exceeding the capacity of the existing DWS without any added growth. In the absence of a capacity expansion no additional development could be approved.



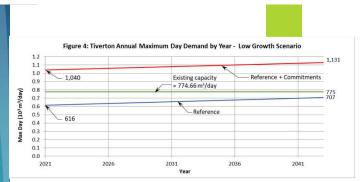


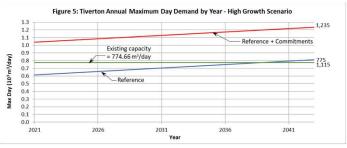
Rated capacity 775 m³/day (restricted by Permit to Take Water) Current maximum demand is 616 m³/day for 372 customers (1.66 m³/day per customer) Current usage is 80% of capacity. Current commitments 256 units or 424 m³/day Uncommitted reserve 265 m³/day or 160 ERU For reference, the current maximum day demand is less than the value of 659 m³/day reported in the 2018 Master Plan

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Tiverton Water Treatment – Forecasted Demand

- Based on the development ERUs considered, the system is over committed. However:
 - ▶ Under the low growth scenario, capacity will be adequate for the entire 20 year period considered.
 - Under the high growth scenario, capacity will be adequate until approximately 2039.
- ► Therefore, actual timing of growth will be a key consideration.





Facility	Total Volume (m³)	Effective Volume (m³)
Kincardine WTP Reservoir	4,120	1,700
Kincardine Standpipe	3,360	3,005
Kincardine Totals	7,480	4,705

Scenario	Total Volume Required (m ³)
Existing	4,650
Existing + Commitments	5,789
Existing + Commitments + Bruce Power	6,650

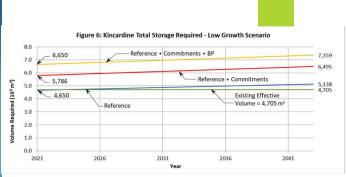
- Storage is used for equalization of peak demands, fire protection, and emergencies
- ► Currently there is sufficient water storage
- Modifications at the WTP could increase total effective volume to 7,090 m³
 - Would be sufficient for commitments + Bruce Power
 - Modifications being examined as part of Bruce Power Water Supply EA

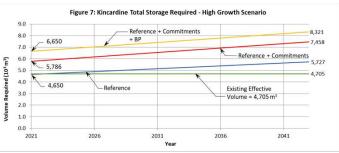
Water Storage -Kincardine

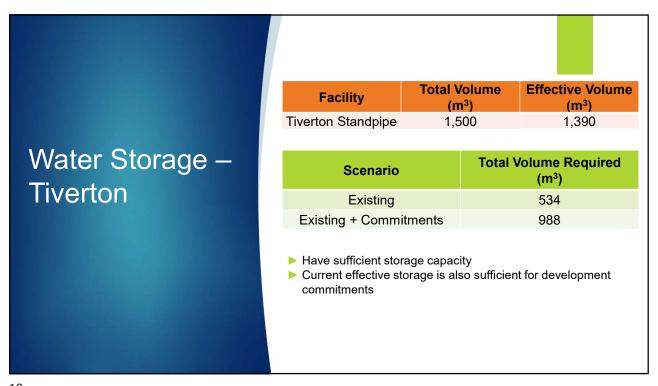
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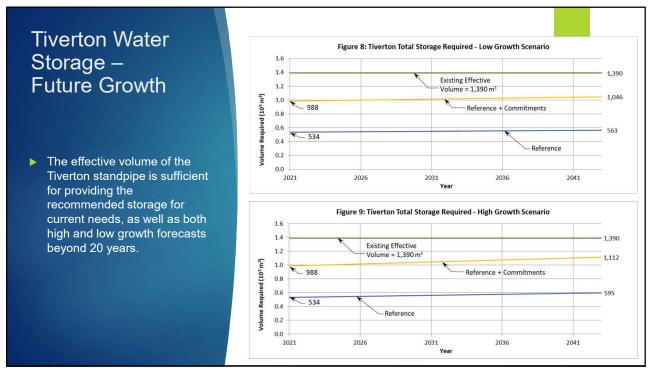
Kincardine Water Storage – Future Growth

- Current effective volume is slightly above recommended value. Under either growth scenario, the recommended volume will exceed effective volume within several years.
- With WTP modifications, storage would be sufficient for existing plus development commitments and Bruce Power needs.









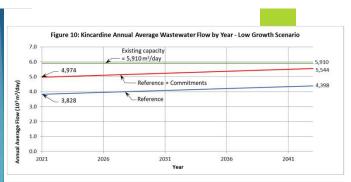
Kincardine WWTP

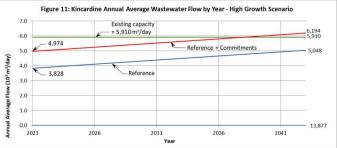
- ▶ Rated capacity is 5,910 m³/day
- Current average flow 3,828 m³/day for 3,780 customers (1.01 m³/day per customer)
- ► Current usage is approximately 65%
- ▶ Committed capacity 1,132 units or 1,146 m³/day
- ▶ Uncommitted reserve capacity 936 m³/day or 924 units
- ► For reference, the current average day flow is similar to the value of 3,811 m³/day reported in the 2018 Master Plan

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Kincardine WWTP – Forecasted Flows

- Under the low growth scenario, the existing rated capacity of the Kincardine WWTP will be sufficient for existing plus development commitments for the next 20 years.
- Under the high growth scenario, the existing rated capacity of the Kincardine WWTP will be fully committed by approximately 2037.



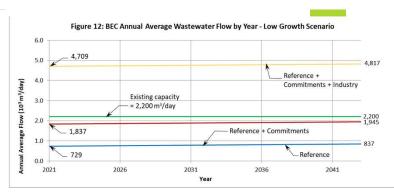


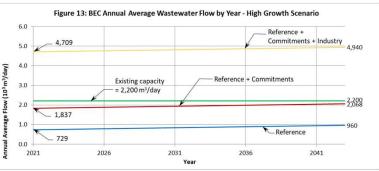
Current rated capacity 2,200 m³/day. Current average flow 729 m³/day by 460 customers (1.58 m³/day per customer) Current usage 33% Current commitments 699 units or 1,108 m³/day **Uncommitted Reserve** 363 m³/day **BEC WWTP** For reference, the current average day flow is less than the value of 805 m³/day reported in the 2018 Master Plan. Additional development within the BEC will need to consider industry specific wastewater servicing needs and associated impacts to reserve capacity. MECP Guideline values for industrial lands would project flows for all vacant BEC lands to exceed the plant capacity, however it is probable that Guideline values are unrealistically high.

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BEC WWTP – Forecasted Flows

- Under both growth scenarios, the existing rated capacity of the BEC WWTP will be sufficient for the next 20 years.
- Addition of BEC and Concession 2 industrial lands has potential to over commit the plant capacity.
- Based on our experience, it is unlikely that all industrial land would be developed with industry for which the shown allowance applies. Ultimately the actual sewage flows generated will be industry specific.





Water Distribution

- ▶ Kincardine and Tiverton distribution systems were each modelled to evaluate predicted pressures and available fire flows, for both existing and 20 year growth scenarios.
- No issues were identified related to available pressure. A limited number of locations have low fire flow; these locations are associated with dead ends and/or at the extremities of the distribution systems.
- Water supply to Bruce Power would result in the need for a booster pumping station (BPS) along the Lakeshore watermain. Identification of a preferred location is part of the current Class EA for the water supply expansion.

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Sewer Collection

▶ Six major Sewage Pumping Station (SPS) catchment areas in Kincardine, and two in Tiverton, were modelled.

Community	Total Number of Pipes in Models (All Catchment Areas)	Number of Pipes at >100% Capacity for Existing Development	Number of Pipes at >100% Capacity for Future Development
Kincardine	705	5 (0.7%)	35 (5%) ¹
Tiverton	101	0	0

- In general, no significant capacity issues of urgent need. Replacement of undersized sewer for future development should be timed for development.
- 1. Park St. SPS catchment area is still under review.

Sewage Pumping Stations

SPS	Rated Capacity (L/s)	Estimated Current Peak (L/s)	Projected 20 Year Peak (L/s)
In Kincardine			
Connaught Park	89	38	60
Durham St.	27 ¹	55	120
Goderich St.	46	61 ³	63
Huron Terrace	300 ²	179	310
Kincardine Ave.	49	40	73
Park St.	99	53	~2005
In Tiverton			
King St.	14	6	12
Maple St.	30	53 ⁴	67

- 1. Durham St. SPS expansion under design, for 2023 tender.
- 2. Huron Terrace SPS expansion is under construction.
- 3. Despite estimated peak > rated capacity, no bypasses in last 3 years.
- 4. Flows appear to have subsided in 2022.
- 5. Park St. SPS catchment area is still under review.

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Alternative Solutions -Water

▶ In general, Limit Growth and Do Nothing are always alternatives. Additionally:

Kincardine	Tiverton	
Water Treatment	Water Treatment	
Increase WTP Capacity	Pursue PTTW increase	
New WTP	Additional well	
Water Storage	Connect to Kincardine system	
WTP disinfection modifications		
New storage facility		
Water Distribution – upgrades in response to development needs		

Alternative Solutions -Wastewater

▶ In general, Limit Growth and Do Nothing are always alternatives. Additionally:

Kincardine	BEC/Tiverton
Wastewater Treatment	Wastewater Treatment
Increase WWTP Capacity	Increase WWTP Capacity
New WWTP	New WWTP
Monitor flows	Monitor flows
Sewage Pumping	Sewage Pumping
 Durham St., Park St. a. Larger pumps and/or forcemain b. New station 	Maple St. a. Monitor flows
Goderich St. Monitor flows	
Sewer Collection – upgrades in	n response to development needs

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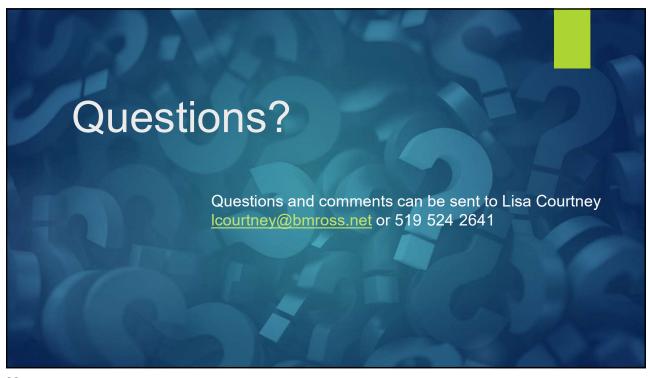
Recommended Solutions - Water

Facility	Recommended Solution	Timing/Class EA Notes
Kincardine WTP	Increase WTP Capacity	Class EA is underway
Kincardine water storage	WTP disinfection modifications	Under review as part of Class EA process
Tiverton water treatment	Further study of additional well vs. connection to Kincardine	Demands have not increased in 5 years, and system has significant storage, therefore no apparent urgency. Recommend to regularly update reserve capacity calculations. New well or connection to Kincardine will require Schedule B Class EA.
Water distribution – both communities	Upgrades as needed for new development	In response to development timing

Recommended Solutions - Wastewater **Timing/Class EA/Other Notes** Flows have not changed significantly in 5 Kincardine WWTP years. Recommend to regularly update Increase WWTP Capacity **BEC WWTP** reserve capacity calculations. Capacity increase will require Schedule C Class EA. Design of Durham station pump upgrades Durham St. and Park St. Larger pumps (short-term) for 2023 construction, and Park for 2024. SPS Larger forcemain (long-term) Forcemain replacement at later date when required. Ongoing. Despite estimated peaks greater Goderich St. and Maple Monitor flows than station capacity, flows/bypasses have subsided. Wastewater collection -Upgrades as needed for new In response to development timing Kincardine development

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Incorporate feedback received from this meeting into draft Master Plan Update document Present draft Master Plan to Council Issue Notice of Completion 30 day public review period





Ministry of the Environment, Conservation and Parks

Ministère de l'Environnement, de la Protection de la nature

et des Parcs

Environmental Assessment

Branch

Direction des évaluations environnementales

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Via E-mail Only

May 31, 2023

Lisa Courtney
Environmental Planner
B. M. Ross and Associates Limited
lcourtney@bmross.net

Re: 2022 Water and Wastewater Servicing Master Plan Update
Municipality of Kincardine
Municipal Class Environmental Assessment – Master Plan (Approach #1)
Project Review Unit Comments – Draft Master Plan Update

Dear Lisa Courtney,

Thank you for providing the ministry with an opportunity to comment on the draft Master Plan Update Report (Report) for the above noted Class Environmental Assessment (EA) project. Our understanding is that in order to address impacts to the water and wastewater systems from local growth and development, the Municipality of Kincardine (the proponent) has determined that the preferred alternatives include: expanding the Kincardine Water Treatment Plant (WTP) capacity; Kincardine WTP disinfection modifications, to increase the effective available volume of water from the existing reservoir; further study of well improvements, an additional well, or connection to Kincardine for the Tiverton Drinking Water System; Durham Street Sewage Pumping Station (SPS) pump upgrades and related works; Park Street SPS pump upgrades and related works; regular updates of the reserve capacity calculations; and sewer upgrades as needed for new development. The Ministry of the Environment, Conservation and Parks (ministry) provides the following comments for your consideration.

General

- 1) Section ES 2.1 of the Report references ERUs, which are defined later in the Report as Equivalent Residential Units (ERUs). The ministry recommends that acronyms in the Report be defined at the first instance of their use.
- 2) Given that Section 1.3.2 of the Report describes the project schedules outlined in the 2023-amended version of the Municipal Class EA (MCEA), the ministry recommends that Figure 1.1, which references Schedule A/A+ projects from the 2015-amended MCEA, be replaced by the equivalent figure "Exhibit A.2" in the 2023-amended MCEA.

Evaluation of Alternatives

3) Section 9.0 of the Report states, "Alternative solutions to the above-noted problems and opportunities were evaluated. Based on the evaluations undertaken, the following solutions were recommended." I note that alternative solutions are not very clearly identified and evaluated for each of the problems/opportunities presented in the report, and the traceability of decision-making for how the recommended/preferred solutions were evaluated and selected could be improved. However, the ministry acknowledges that this Approach #1 Master Plan consists of only preliminary completion of Phases 1 and 2 of the Municipal Class EA process, rather than a fulsome completion of these phases. Please ensure that subsequent Schedule B and C projects that are supported by this Master Plan complete the entirety of the applicable phases of the Municipal Class EA process, including identification of all reasonable and feasible alternative solutions to the problem, identification of the magnitude of the net positive and negative effects of each alternative solution and mitigating measures, and an evaluation of all reasonable alternative solutions.

Thank you for circulating this draft Report for the ministry's consideration. Please document the provision of the draft Report to the ministry as well as this Project Review Unit Comments letter in the final Report. A copy of the final Notice of Master Plan should be sent to the ministry's Southwest Region EA notification email account (eanotification.swregion@ontario.ca).

Should you or any members of your project team have any questions regarding the material above, please contact me at mark.badali1@ontario.ca.

Sincerely,

CC

Mark Bedeli

Mark Badali, Senior Project Evaluator

Environmental Assessment Program Support, Environmental Assessment Branch Ontario Ministry of the Environment, Conservation and Parks